

DRAWING LIST		
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S020	TYPICAL DETAILS 1	G
S021	TYPICAL DETAILS 2	G
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CLIENT

CITY OF TORONTO



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PROJECT TITLE
TORONTO PARAMEDIC
SERVICES FLEET
MAINTENANCE STATION -
UPGRADE WORKS

PROJECT ADDRESS

KING STREET YARD -
BUILDING NO. 8 & 9
1116 KING STREET WEST

PROJECT NO:
30276606

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SHEET TITLE
DRAWING LIST

SHEET NUMBER

S001

ISSUE

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GENERAL:

- DO NOT SCALE THE DRAWINGS.
- THE DRAWINGS ARE TO BE READ IN CONJUNCTION WITH THE SPECIFICATIONS, ALL RELATED ARCHITECTURAL, MECHANICAL, ELECTRICAL, PROCESS & CIVIL DRAWINGS, AND OTHER RELEVANT CONTRACT DOCUMENTS.
- CONTRACTOR SHALL PROVIDE COMPLETE SET OF ARCHITECTURAL, MECHANICAL, ELECTRICAL, CIVIL & PROCESS DRAWINGS & APPLICABLE SPECIFICATION SECTIONS TO THE STRUCTURAL STEEL, JOIST & METAL FABRICATIONS CONTRACTORS PRIOR TO SUBMISSION OF ANY RELATED SHOP DRAWINGS.
- THE DESIGN AND CONSTRUCTION OF ALL WORK ON THIS PROJECT IS TO CONFORM TO THE 2024 EDITION OF THE ONTARIO BUILDING CODE.
- THE CONTRACTOR SHALL FIELD CHECK AND VERIFY ALL DIMENSIONS, ELEVATIONS AND CONDITIONS AT THE SITE AND REPORT TO THE ENGINEER ANY DISCREPANCIES OR UNSATISFACTORY CONDITIONS WHICH MAY ADVERSELY AFFECT THE PROPER COMPLETION, COST, SCHEDULE OR QUALITY OF WORK. COMMENCEMENT OF WORK BY THE CONTRACTOR IMPLIES ACCEPTANCE OF THE EXISTING CONDITIONS.
- THE CONTRACTOR SHALL PROTECT ALL EXISTING STRUCTURES, UNDERGROUND UTILITIES AND OTHER EXISTING SERVICES DURING CONSTRUCTION, AND MAKE GOOD ANY DAMAGE RESULTING FROM THE WORK ON THIS PROJECT TO THE SATISFACTION OF THE OWNER AND ENGINEER.
- THE CONTRACTOR SHALL PROVIDE, MAINTAIN AND TAKE RESPONSIBILITY FOR ALL TEMPORARY BRACING AND SHORING.
- TYPICAL DETAILS SHOWN ON DRAWINGS SHALL GOVERN THE WORK. IF DETAILS DIFFER ON OTHER DRAWINGS, THE MOST STRINGENT SHALL GOVERN.
- WORK NOT INDICATED ON A PART OF THE DRAWINGS BUT REASONABLY IMPLIED TO BE SIMILAR TO THAT SHOWN AT CORRESPONDING PLACES SHALL BE REPEATED.

CONCRETE:

- THE DESIGN AND CONSTRUCTION OF ALL WORK ON THIS PROJECT SHALL CONFORM TO THE APPLICABLE EDITION OF THE FOLLOWING CSA STANDARDS:
CSA-A23.1 "CONCRETE MATERIALS AND METHODS OF CONCRETE CONSTRUCTION",
CSA-A23.2 "TEST METHODS AND STANDARD PRACTICES FOR CONCRETE",
CSA-A23.3 "DESIGN OF CONCRETE STRUCTURES",
CAN/CSA-G30.18 "BILLET-STEEL BARS FOR CONCRETE REINFORCEMENT".
- UNLESS NOTED OTHERWISE, MINIMUM CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS SHALL BE AS FOLLOWS:

- LEAN CONCRETE FILL 15 MPa
- MUD SLAB 5 MPa (EXPOSURE CLASS F-1)
- FOOTINGS 35 MPa (EXPOSURE CLASS C-1)
- PIERS, WALLS & CURBS 35 MPa (EXPOSURE CLASS C-1)
- INTERIOR SLABS ON GRADE 30 MPa (EXPOSURE CLASS C-1)
SEE FOUNDATION AND SLAB ON GRADE PLANS AND SPEC.
- MINIMUM CONCRETE COVER TO REINFORCING BARS:

CONCRETE CLEAR COVER (CSA A23.1			
EXPOSURE CONDITION	EXPOSURE CLASS		
	N	F-1	C-1
CAST AGAISNT AND PERMANENTLY EXPOSED TO EARTH, INCLUDING FOOTINGS AND PILES	75 mm	75 mm	75 mm
BEAMS, GIRDERS, COLUMNS AND PILES	30 mm (#1)	40 mm	60 mm
SLABS, WALLS, JOISTS, SHELLS AND FOLDED PLATES	20 mm (#1)	40 mm	60 mm
RATIO OF COVER TO NOMIMAL BAR DIAMETER (#2)	1.0 (#1)	1.5	2.0
RATIO OF COVER TO NOMIMAL MAXIMUM AGGREGATE SIZE	1.0 (#1 & #3)	1.5	2.0

- NOTES:
- THIS REFERS ONLY TO CONCRETE THAT WILL BE CONTINUALLY DRY WITHIN THE CONDITIONED SPACE (i.e., MEMBERS ENTIRELY WITHIN THE VAPOUR BARRIER OF THE BUILDING ENVELOPE).
 - THE COVER FOR A BUNDLE OF BARS SHALL BE THE SAME AS THAT FOR A SINGLE BAR WITH AN EQUIVALENT AREA.
 - THE SPECIFIED COVER FROM SCREEDED SURFACES SHALL BE AT LEAST 1.5 TIMES THE NOMINAL MAXIMUM AGGREGATE SIZE TO REDUCE INTERFERENCE BETWEEN AGGREGATE AND REINFORCEMENT WHERE VARIATIONS IN BAR PLACEMENT RESULT IN A COVER SMALLER THAN SPECIFIED.
 - DETAIL, BEND, PLACE AND SUPPORT REINFORCING STEEL IN CONFORMANCE WITH THE LATEST RSIC MANUAL OF STANDARD PRACTICE, UNLESS NOTED OTHERWISE.
 - ALTERNATE 90° HOOKS OF CROSSTIES END FOR END OVER HEIGHT OF ALL MEMBERS IN WHICH CROSSTIES ARE SPECIFIED.
 - ALL LAP SPLICES SHALL BE CLASS B TENSION SPLICES. FOR HORIZONTAL REINFORCEMENT THAT WILL BE PLACED IN SUCH A WAY THAT MORE THAN 300 mm OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE SPLICE, BAR LOCATION FACTOR (k/1) EQUAL TO 1.3 SHALL BE APPLIED IN DETERMINING CLASS B TENSION SPLICE LENGTH, IN ACCORDANCE WITH APPLICABLE EDITION OF CSA-A23.3.
 - PLAIN (UNREINFORCED) CONCRETE EXPOSED TO DEICING CHEMICALS SHALL MEET EXPOSURE CLASS C-2 IN ACCORDANCE WITH APPLICABLE EDITION OF CSA-A23.1.
 - REINFORCED CONCRETE EXPOSED TO CHLORIDES SHALL MEET EXPOSURE CLASSIFICATION C-1 IN ACCORDANCE WITH APPLICABLE EDITION OF CSA-A23.1.
 - PROVIDE CORROSION INHIBITOR IN ALL CONCRETE OF EXPOSURE CLASS C-1. REFER TO SPECIFICATIONS FOR CORROSION INHIBITOR DOSAGE RATE.
 - USE TYPE GU PORTLAND CEMENT FOR ALL CONCRETE, UNLESS NOTED OTHERWISE.
 - ALL HORIZONTAL WALL AND GRADE BEAM REINFORCING SHALL BE CONTINUOUS THROUGH PIERS, UNLESS NOTED OTHERWISE, SPLICE TOP BARS AT MID-SPAN AND BOTTOM BARS AT SUPPORTS FOR ALL GRADE BEAMS.
 - WHEREVER OPENINGS OCCUR, INTERRUPTING ONE OR MORE REINFORCING BARS, IN SLABS OR WALLS, PROVIDE ADDITIONAL REINFORCING STEEL EQUAL TO THE REINFORCING STEEL DISPLACED BY THE OPENING UNLESS OTHERWISE SHOWN. DISTRIBUTE REINFORCEMENT EQUALLY ON EACH SIDE OF THE OPENING AND EXTENDING THE FULL SPAN LENGTH.
 - NO SLEEVES, PIPES, HOLES OR NOTCHES SHALL BE PLACED IN OR THROUGH WALLS, COLUMNS, PIERS, BEAMS, GRADE BEAMS, SLABS, FOOTINGS, OR ANY OTHER CONCRETE MEMBER, EXCEPT AS SHOWN ON THE STRUCTURAL DRAWINGS OR APPROVED BY THE ENGINEER.
 - DO NOT PLACE CONCRETE FOOTINGS OR ANY OTHER CONCRETE MEMBER UNTIL ALL DESIGNATED REINFORCING STEEL HAS BEEN PLACED AND INSPECTED AND ANY CONDUITS, PIPING OR OTHER EMBEDDED ITEMS ARE INSTALLED AND VERIFIED.
 - MAXIMUM WATER-TO-CEMENTING MATERIALS RATIO (w/cm) SHALL BE 0.55, AND MINIMUM 28-DAY COMPRESSIVE STRENGTH SHALL BE 30 MPa FOR ALL CONCRETE FLOOR SLABS (BOTH SLABS ON GRADE AND SLABS ON DECK).

SLAB ON GRADE:

- PLACE SLAB ON GRADE ON OPSS GRANULAR 'A' COMPACTED TO 100% STANDARD PROCTOR MAXIMUM DRY DENSITY. MINIMUM SUBGRADE MODULUS k = 34 MN/m3.
- ALL THICKENED SLABS SHALL BEAR ON NATIVE MATERIAL OR ENGINEERED FILL WITH MINIMUM SLS BEARING RESISTANCE OF 100 kPa AND MINIMUM ULS BEARING RESISTANCE OF 150 kPa.
- PRIOR TO PLACING GRANULAR FILL MATERIALS, PROOF-ROLL EXISTING SUB-GRADE TO IDENTIFY INCONSISTENCIES OR SOFT AREAS, PROCEED WITH GRANULAR PLACEMENT ONLY AFTER THESE AREAS HAVE BEEN REWORKED.
- DO NOT POUR CONCRETE UNTIL ALL EMBEDDED STEEL, ELECTRICAL AND MECHANICAL CONDUITS, PIPING OR OTHER EMBEDDED SERVICES ARE INSTALLED AND VERIFIED.
- PROVIDE CONSTRUCTION, CONTROL AND ISOLATION JOINTS AS SHOWN ON DETAILS.
- PERFORM SAW CUTTING FOR CONTROL JOINTS USING A "SOFF-CUT" SAW. SPACING TO MATCH EXISTING OR 6000 mm MAXIMUM.
- SLAB ON GRADE FINISH TO MATCH EXISTING.
- PROVIDE 2-15M x 1200 mm LONG DIAGONAL TOP BARS AT ALL RE-ENTRANT CORNERS AROUND GRADE WALLS, PITS AND SLAB HOLD DOWNS.
- REFER TO ARCHITECTURAL DRAWINGS FOR LOCATION AND DETAILS OF SLAB HOLD DOWNS FOR FLOOR FINISHES AND SLOPED FLOOR PROFILES AROUND FLOOR DRAINS, SUMPS, ENTRY DOORS, ETC.
- REFER TO MECHANICAL DRAWINGS FOR SIZE AND LOCATION OF EXISTING UNDER SLAB PIPING AND OTHER UTILITIES AND STRUCTURES TO REMAIN OR TO BE ABANDONED IN-PLACE.
- REFER TO MECHANICAL AND ELECTRICAL DRAWINGS FOR SIZE AND LOCATION OF UNDER SLAB AND EMBEDDED PIPING, PLUMBING, FLOOR DRAINS, DUCTS, CONDUIT, ETC.
- DO NOT LOAD SLABS ON GRADE WITH ERECTION CRANES OR ERECTION EQUIPMENT. SLABS HAVE NOT BEEN DESIGNED FOR CRANE LOADS AND WILL REQUIRE TEMPORARY CRIBBING OR OTHER MEANS TO DISTRIBUTE CONCENTRATED LOADS. OBTAIN ENGINEER'S APPROVAL ON PROPOSED CRANE SUPPORT PLAN FOR SLABS PRIOR TO COMMENCING WORK.
- CONTRACTOR SHALL PERFORM SUB-BASE TESTING PER SPECIFICATION SECTION 1400. THIS TESTING SHALL BE DONE IN ADVANCE AND PROVIDED TO ARCADIS ENVIRONMENTAL ENGINEER AND ARCADIS STRUCTURAL ENGINEER FOR REVIEW AND APPROVAL PRIOR TO START OF WORK.
- UNLESS NOTED OTHERWISE, MINIMUM CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS SHALL BE 30 MPa (EXPOSURE CLASS N-CF).

MASONRY:

- THE DESIGN AND CONSTRUCTION OF ALL WORK ON THIS PROJECT IS TO CONFORM TO APPLICABLE EDITION OF CSA STANDARDS CSA-S304.1, CSA-A370, CSA-A371, CSA-A179 AND CAN/CSA-G30.18. PROVIDE TYPE S MORTAR IN ACCORDANCE WITH APPLICABLE EDITION OF CSA STANDARD CSA-A179.
- PROVIDE STANDARD, HOLLOW 15 MPa CONCRETE BLOCK UNITS UNLESS NOTED OTHERWISE. AT LOCATIONS WHERE VERTICALLY-REINFORCED MASONRY IS SPECIFIED, PROVIDE ONE ADDITIONAL VERTICAL BAR OF SIZE EQUAL TO DISTRIBUTED VERTICAL REINFORCING, FULL HEIGHT AT EACH SIDE OF CONTROL JOINTS, AND AT CORNERS, INTERSECTIONS, ENDS OF WALLS AND TO EACH SIDE OF ALL OPENINGS, UNLESS NOTED OTHERWISE. PROVIDE 2 ADDITIONAL BARS WHERE OPENING WIDTH IS GREATER THAN 1200 mm. PROVIDE MATCHING DOWELS TO FOOTING, SLAB THICKENING, SLAB ON DECK OR OTHER SUPPORTING STRUCTURE, FOR ALL VERTICAL WALL REINF. (TYP. UIN).
- FOR HIGH-LIFT GROUTING, PROVIDE CLEAN-OUT HOLES IN BOTTOM COURSE AND MINIMUM SLUMP OF 200 mm.
- MASONRY GROUT SHALL BE FINE GROUT, AND SHALL BE PROPORTIONED ACCORDING TO, AND MEET ALL REQUIREMENTS OF APPLICABLE EDITION OF CSA-A179.
- PROVIDE CONTINUOUS, HORIZONTAL STANDARD LADDER-TYPE HORIZONTAL JOINT REINFORCEMENT @ 400 mm C/C, IN BOTTOM TWO BED JOINTS, TOP TWO BED JOINTS AND FIRST BED JOINT ABOVE AND BELOW ALL WALL OPENINGS. ALL SUCH REINF. SHALL BE SPLICED USING CLASS B TENSION LAP SPLICES (AS DEFINED IN APPLICABLE EDITION OF CSA-S304.1). PREFABRICATED CORNER AND INTERSECTION JOINT REINFORCING PIECES SHALL BE USED. MINIMUM WIRE DIAMETER TO BE 3.65 mm.
- REFER TO ARCHITECTURAL DRAWINGS FOR LOCATION OF ALL MASONRY WALLS.
- PROVIDE VERTICAL REINFORCING IN ALL MASONRY WALLS AS PER TABLE ON DRAWING S021 ALL REINFORCED CORES SHALL BE GROUTED SOLID FOR FULL HEIGHT OF WALLS. VERTICALLY-CANTILEVERED WALLS SHALL BE CONSTRUCTED OF SOLID-GROUTED MASONRY.
- PROVIDE VERTICAL REINFORCING AT EACH LATERAL SUPPORT LOCATION.
- LAP ALL 15M BARS 675 mm MINIMUM. LAP ALL 20M BARS 850 mm MINIMUM.
- UNLESS NOTED OTHERWISE, MINIMUM CLEARANCE OF XX mm SHALL BE PROVIDED BETWEEN MASONRY WALLS AND STEEL COLUMNS, BEAMS, GIRDERS, JOIST AND GIRTS. THIS REQUIREMENT DOES NOT APPLY WHERE MASONRY IS INTENDED (BY DESIGN) TO BEAR ON STEEL MEMBER.
- WHERE POST-INSTALLED ANCHORS ARE INSTALLED IN MASONRY ELEMENTS/MEMBERS, EXISTING REINFORCING STEEL (INCLUDING HORIZONTAL BED JOINT REINFORCING), CAST-INS AND EMBEDDED ITEMS SHALL BE ACCURATELY LOCATED, AND EDGE OF ANCHOR HOLES SHALL BE LOCATED A DISTANCE AWAY FROM EXISTING REINFORCING STEEL (INCLUDING HORIZONTAL BED JOINT REINFORCING), CAST-INS END EMBEDDED ITEM SUCH THAT AT LEAST MINIMUM CONCRETE COVER REQUIREMENTS (SEE MASONRY NOTE #1) ARE MET. EXISTING REINFORCING STEEL (INCLUDING HORIZONTAL BED JOINT REINFORCING), CAST-INS AND EMBEDDED ITEMS SHALL NOT BE DAMAGED ANCHORING ACTIVITIES, INCLUDING DRILLING.
- TIMBER:
- ALL WOOD MATERIALS, FABRICATION AND ERECTION SHALL BE IN ACCORDANCE WITH APPLICABLE EDITION OF CAN/CSA-086. ALL TIMBER GRADING TO BE IN ACCORDANCE WITH NLGA.
- ALL TIMBER FOR TRUSSES, LINTELS, BEAMS, AND STUDS TO BE S-P-F NO.1/NO.2 UNLESS OTHERWISE NOTED ON DRAWINGS.
- ALL TIMBER SHALL CONFORM TO RECOGNIZED NOMINAL SIZES SHOWN ON PLAN AND STRESS RATING FOR APPROPRIATE SPECIES. NO TIMBER SHALL BE USED THAT DOES NOT CONFORM TO DIMENSIONS AND SPECIES.
- ALL WALLS AND ROOFS SHALL BE FULLY BLOCKED.
- MAXIMUM BLOCKING SPACING: WALLS AND ROOFS = 1200 mm ± TO MATCH SHEATHING JOINTS.
- FRAMING INTO FLUSH BEAMS SHALL BE FASTENED WITH STEEL JOIST OR BEAM HANGERS.
- SPIKE ALL BUILT-UP SAWN LUMBER BEAMS @ 300 mm C/C IN ROWS NOT EXCEEDING 75 mm C/C.
- SPIKE & GLUE BUILT-UP STUDS AND COLUMNS @ 150 mm C/C IN ROWS NOT EXCEEDING 75 mm C/C.
- ALL PLYWOOD SHEATHING SHALL BE FASTENED TO WOOD FRAMING WITH COMMON WIRE NAILS.
- ALL WOOD IN CONTACT WITH CONCRETE SHALL BE PRESSURE TREATED.
- PREMANUFACTURED SPECIALTY WOOD PRODUCTS SUCH AS 'PARALLAM' PARALLEL STRAND LUMBER, 'MICROLLAM' LAMINATED VENEER LUMBER, 'TIMBERSTRAND' LAMINATED STRAND LUMBER, WOOD I OR TRUSS-JOIST MANUFACTURED I JOISTS, MUST BE STORED, HANDLED, MODIFIED AND ERECTED AS PER MANUFACTURER'S DETAILED DIRECTIONS. AN INDEPENDENT INSPECTION & TESTING COMPANY RETAINED BY THE MANUFACTURER WILL PROVIDE PRODUCT CERTIFICATION DOCUMENTS.
- ALL FASTENERS, ANCHORS AND CONNECTIONS SHALL BE HOT-DIP GALVANIZED.

STRUCTURAL STEEL:

- ALL STRUCTURAL STEEL MEMBERS ARE DESIGNED IN ACCORDANCE WITH CAN/CSA-S16, "DESIGN OF STEEL STRUCTURES".
- STRUCTURAL STEEL SHALL CONFORM TO CSA STANDARD CAN/CSA-G40.20/G40.21-13, GRADE 350W FOR ROLLED SHAPES AND GRADE 350W CLASS C FOR HSS SECTIONS UNLESS NOTED OTHERWISE.
- ANGLES, PLATES AND CHANNELS (L, C) SHALL CONFORM TO CAN/CSA-G40.20/G40.21 GRADE 300W, UIN.
- UNLESS OTHERWISE INDICATED, ALL BOLTS SHALL BE HIGH TENSILE STEEL CONFORMING TO ASTM A325 DESIGN REQUIREMENTS EXCEPT FOR ANCHOR RODS. BOLTED CONNECTIONS SHALL BE BEARING TYPE EXCEPT WHERE CONNECTIONS ARE REQUIRED TO RESIST AXIAL FORCES IN WHICH CASE SLIP CRITICAL CONNECTIONS SHALL BE USED. BOLTS SHALL BE INSTALLED TO A SNUG-TIGHT CONDITION AS DEFINED IN CSA S16.
- ANCHOR RODS SHALL CONFORM TO ASTM F1554 GRADE 55 WELDABLE, UNLESS NOTED OTHERWISE ON THE DRAWINGS.
- ALL WELDING SHALL CONFORM TO CSA STANDARD W48-14 AND W59-13 AND SHALL BE UNDERTAKEN BY A FABRICATOR QUALIFIED IN ACCORDANCE WITH CSA STANDARD W47.1-09 (R2014).
- NON-SHRINK GROUT SHALL HAVE A 45 MPa MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS, UNLESS OTHERWISE NOTED OR SHOWN
- CORROSION PROTECTION SHALL BE AS PER SPECIFICATIONS UNLESS NOTED OTHERWISE ON DRAWINGS. STEEL FINISH TO BE AS PER SPECIFICATION SECTION 05 12 00 2.1.9.
- PROVIDE MINIMUM LENGTH OF BEARING OF 200 mm FOR ALL STEEL BEAMS BEARING ON MASONRY AND CONCRETE AND A MINIMUM OF 100 mm ON STRUCTURAL STEEL, UNLESS OTHERWISE NOTED OR SHOWN.
- CENTRE BEARING PLATES UNDER BEAMS UNLESS OTHERWISE NOTED OR SHOWN.
- NO STRUCTURAL STEEL SHALL BE CUT IN THE FIELD UNLESS REVIEWED AND APPROVED BY THE DESIGN ENGINEER.
- ALL WELDS EXPOSED TO VIEW SHALL BE GROUND SMOOTH.
- IT IS THE INTENTION OF THESE DESIGN DOCUMENTS TO DELEGATE THE DESIGN OF ALL STRUCTURAL STEEL CONNECTIONS TO THE STEEL FABRICATOR. THE STEEL FABRICATOR SHALL RETAIN A PROFESSIONAL ENGINEER, LICENSED IN THE PROVINCE OF ONTARIO TO PERFORM ALL DESIGN CALCULATIONS. THIS REQUIREMENT EXTENDS TO ALL CONNECTIONS. IT IS ANTICIPATED THAT PROSPECTIVE STRUCTURAL STEEL FABRICATORS WILL PERFORM NECESSARY INVESTIGATION TO DETERMINE THE FULL IMPACT OF CONNECTION CLEARANCE REQUIREMENTS, AS WELL AS THE POTENTIAL NECESSARY INTRODUCTION OF DOUBLER PLATES, CONTINUITY PLATES, AND/OR WEB FLANGE OR OTHER STIFFENERS PRIOR TO SUBMITTING ANY BID FOR THIS WORK. CONNECTIONS SHOWN ON DRAWINGS, INCLUDING WELDS, BOLTS AND STIFFENERS ARE FOR INDICATIVE PURPOSES ONLY AND REPRESENT MINIMUM REQUIREMENTS.
- PROVIDE SIGNED AND SEALED DRAWINGS AND CALCULATIONS FOR ALL STRUCTURAL STEEL CONNECTION DESIGN. CALCULATIONS ARE TO BE SUBMITTED SIMULTANEOUSLY WITH CORRESPONDING SUBMITTAL. DO NOT PROCEED WITH FABRICATION UNTIL SUBMITTAL HAS BEEN REVIEWED BY STRUCTURAL ENGINEER OF RECORD.
- SHAPE AND SIZE GUSSET PLATES TO CLEAR ARCHITECTURAL FINISHES, MECHANICAL DUCTS, PIPES, ELEVATOR SHAFTS AND THE LIKE.
- PROVIDE CAMBER TO BEAMS, GIRDERS AND TRUSSES AS SHOWN ON THE PLANS. CAMBERS SHOWN ARE FOR ERECTED IN-PLACE CONDITION OF MEMBERS BEFORE INSTALLATION OF STEEL DECK. WHERE CONCRETE ON STEEL DECK IS CALLED FOR, SCREED SLAB TO SUIT BEAM CAMBERS.
- PROVIDE SLOTTED HOLES AND SLIP-CRITICAL BOLTED CONNECTIONS TO CONNECT NEW STEEL TO EXISTING WORK.
- UNLESS OTHERWISE SHOWN ON THE DRAWINGS, TYPICAL SHEAR CONNECTIONS ARE TO BE DESIGNED AS FOLLOWS:
i) TYPICAL BEAM TO BEAM (NON-SPANDREL CONDITION) CONNECTIONS - ONE SIDED, TWO SIDED OR END PLATE CONNECTIONS.
ii) TYPICAL BEAM TO SPANDREL BEAM CONNECTIONS - TWO SIDED OR END PLATE CONNECTIONS.
iii) TYPICAL BEAM TO COLUMN CONNECTIONS - TWO SIDED OR END PLATE CONNECTIONS.
iv) TYPICAL SPANDREL BEAM TO COLUMN CONNECTIONS - TWO SIDED CONNECTIONS.
v) ALL OTHER CONDITIONS - TWO SIDED OR END PLATE CONNECTIONS.

NOTE: THE ADDITION OF OTHER CONNECTION FORCES (I.E. AXIAL FORCES) MAY RESULT IN DIFFERENT CONNECTIONS TO THOSE NOTED ABOVE.
- THE CONTRACTOR SHALL COORDINATE EMBEDDED STRUCTURAL STEEL ITEMS REQUIRED FOR ARCHITECTURAL, STRUCTURAL, AND MECHANICAL ELEMENTS.
- WELDS SHOWN ON STRUCTURAL DRAWINGS ARE MINIMUM DESIGN REQUIREMENTS. THE FABRICATOR'S SHOP DRAWINGS SHALL REFLECT WELDS IN ACCORDANCE WITH CWS REQUIREMENTS.
- FULL PENETRATION GROOVE WELDS SHALL BE INSPECTED BY ULTRASONIC TESTING.
- TWENTY- FIVE PERCENT OF THE WELDS SHALL BE INSPECTED AT RANDOM UNLESS NOTED OTHERWISE. IF WELDS ARE FOUND TO BE DEFICIENT, ADDITIONAL TESTING SHALL BE PERFORMED AS DIRECTED BY PROJECT CO.
- UNLESS NOTED OTHERWISE ON THE DRAWINGS, GROOVE WELDS SHALL BE FULL PENETRATION.
- CONTRACTOR SHALL BE RESPONSIBLE FOR VISITING THE SITE DURING THE TENDER TO IDENTIFY ANY INTERFERENCE THAT WOULD PREVENT ERECTION OR REINFORCEMENT OF STRUCTURAL STEEL. ANY INTERFERENCE SHALL TO BE RELOCATED AS REQUIRED SO AS TO PERMIT ERECTION OR REINFORCEMENT OF THE STRUCTURAL STEEL. CONTRACTOR SHALL CONSULT MECHANICAL AND ELECTRICAL DRAWINGS AND SPECIFICATIONS TO DETERMINE REQUIREMENTS PRIOR TO RELOCATING ANY INTERFERENCES.
- MINIMUM STIFFENER PLATE THICKNESS SHALL BE 10 mm, UNLESS OTHERWISE NOTED OR SHOWN.

FIBER REINFORCEMENT:

- ALL SLAB ON GROUND SHALL BE REINFORCED WITH STEEL FIBERS, UNLESS OTHERWISE APPROVED BY THE ENGINEER OF RECORD.
- MATERIALS, BATCHING REQUIREMENTS, MIXING AND TESTING PROCEDURE SHOULD COMPLY WITH THE APPLICABLE SECTION OF ASTM C1116.
- PROVIDE MINIMUM DOSAGE OF FIBERS TO ACHIEVE A MINIMUM POST CRACK EQUIVALENT RESIDUAL STRENGTH IN ACCORDANCE WITH ASTM C1609.
- STEEL FIBER REINFORCED CONCRETE (SFRC) SLABS SHALL RECEIVE FIBER REINFORCING AND FIBER DOSAGE PER SPECIFICATION.

REINFORCING STEEL:

- ALL REINFORCING STEEL SHALL CONFORM TO CAN/CSA-G30.18-M-2009 (R2014), GRADE 400W.
- MINIMUM CLEAR SPACING BETWEEN PARALLEL REINFORCING BARS SHALL BE 1.4 TIMES THE BAR DIAMETER, 1.4 TIMES THE MAX. AGGREGATE SIZE OR 30 mm WHICHEVER IS GREATER. PROVIDE 75 mm MINIMUM LAPPED REBAR CLEARANCE FOR CONCRETE VIBRATOR ACCESS.
- DEVELOPMENT LENGTHS AND LAP SPLICES SHALL BE IN ACCORDANCE WITH THE REINFORCING STEEL INSTITUTE OF CANADA, MANUAL OF STANDARD PRACTICE UNLESS INDICATED OTHERWISE. ALL LAP SPLICES SHALL BE CLASS 'B' TENSION LAP SPLICES AND CLASS 'B' TENSION LAP SPLICES SHALL BE PROVIDED FOR ALL SHRINKAGE AND TEMPERATURE BARS, WHERE TWO BARS OF DIFFERENT DIAMETERS ARE LAPPED, PROVIDE CLASS 'B' TENSION LAP LENGTH CORRESPONDING TO THE SMALLER BAR BUT NO LESS THAN FULL TENSION DEVELOPMENT LENGTH OF THE LARGER BAR.
- PROVIDE EMBEDMENT LENGTH FOR FULL TENSION DEVELOPMENT IN ACCORDANCE WITH CSA A23.3-14.
- STANDARD 180° HOOK TO CONFORM TO A23.3 CLAUSE 12.5
- REINFORCING BAR TYING SHALL FOLLOW OPSS 905 (CLAUSE 905.07.02.03 PLACING) FOR MINIMAL CORROSION REQUIREMENT.
- REINFORCING STEEL MUST MEET BEND TEST REQUIREMENTS FOR RESPECTIVE REBAR GRADES PER CSA STANDARD G30.18.
- WHERE REINFORCING BARS ARE SHOWN CONTINUOUS, SPLICE BOTTOM BARS OVER SUPPORTS AND TOP BARS AT CENTRE OF SPAN UIN. ALL OTHER LAP SPLICES SHALL BE IN ACCORDANCE WITH CSA23.3, UIN.
- PROVIDE DOWELS FOR WALLS AND COLUMNS SIMILAR IN NUMBER, SIZE AND SPACING TO THE VERTICAL STEEL IN THE WALL OR COLUMNS ABOVE UNLESS OTHERWISE NOTED OR SHOWN.

METAL DECK

- DESIGN, FABRICATION AND ERECTION OF METAL ROOF DECK, NON-COMPOSITE PERMANENT METAL FORM DECK AND COMPOSITE METAL FLOOR DECK SHALL CONFORM TO THE STEEL DECK INSTITUTE (SDI) CODE OF RECOMMENDED STANDARD PRACTICE AND BASIC DESIGN SPECIFICATIONS".
- ALL METAL DECK SHALL CONFORM TO THE REQUIREMENTS OF CSA S136 UNLESS NOTED OTHERWISE.
- DECK SHALL BE MANUFACTURED FROM STEEL SHEETS CONFORMING TO ASTM A611 GRADE C AND D OR ASTM A653 OR HIGHER SPECIFICATIONS. MINIMUM YIELD STRENGTH SHALL BE 345 MPa.
- INSTALL DECK AS TWO-SPAN CONTINUOUS MINIMUM AND THREE-SPAN CONTINUOUS WHERE POSSIBLE.
- LAP ROOF DECK AND FORM DECK ENDS A MINIMUM OF 50 mm. ENDS OF COMPOSITE FLOOR DECK SHALL BE BUTTED.
- ATTACH DECK TO SUPPORT FRAMING AND PROVIDE SIDE LAPS AS INDICATED AND IN ACCORDANCE WITH THE DECK MANUFACTURER'S INSTRUCTIONS FOR INSTALLATION.
- AT PERIMETER OF DECK, SECURE DECK TO SUPPORT FRAMING WITH SAME ATTACHMENT METHOD AND SPACING AS INDICATED FOR INTERIOR SUPPORTS.
- WHERE PARTIAL PANELS MAY BE REQUIRED TO COMPLETE DECK INSTALLATION AT PERIMETER OF STRUCTURE, PROVIDE ATTACHMENTS IN EACH FLUTE TO SUPPORT FRAMING.
- AT ENDS OF DECK OR WHERE CHANGES OF DECK DIRECTION OCCUR, FASTEN TO SUPPORTS AT EACH FLUTE. PROVIDE ADEQUATE CLOSURES AND FASTENERS TO SIDES AT EIGHTEEN INCHES ON CENTER.
- PROVIDE SIX INCH CLOSURE STRIP WHERE CHANGES IN DECK DIRECTION OCCUR. CLOSURE MATERIAL SHALL BE SAME GAGE AS DECK.
- WHERE DECK IS ATTACHED TO SUPPORT FRAMING BY WELDING, PROVIDE WELDING MATERIALS AND INSTALLATION PROCEDURES TO PREVENT BURNING HOLES OR OTHERWISE DAMAGING DECK.
- MECHANICAL FASTENERS SHALL BE FM APPROVED POWDER-ACTUATED PIN TYPE DECK FASTENERS USE HILTI X-END19 THQ 12M FASTENERS WITH MINIMUM 12MM WASHERS ON STEEL JOISTS AND BEAMS. MAXIMUM SPACING OF MECHANICAL FASTENERS SHALL BE AS FOLLOWS:
- ROOF JOIST FASTENERS AT 300mm C/C (INTERIOR)
- FASTENERS AT 150 mm C/C (PERIMETER AND CORNERS)
- SIDE LAP BUTTON CLINCHED AT 300mm C/C
- ROOF DECK TO BE FASTENED IN ACCORDANCE WITH FM REQUIREMENTS. SUBMIT DRAWINGS SHOWING FASTENING DETAILS FOR REVIEW.
- ALL CONNECTIONS OF ROOF DECK TO SUPPORTING STRUCTURE SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER LICENSED TO PRACTICE IN THE PROVINCE OF ONTARIO TO RESIST ALL LOADS AND EFFECTS SHOWN ON DRAWINGS, BUT SHALL NOT BE LESS THAN THAT SHOWN ON DRAWINGS.

CLIMATE DESIGN LOADS:

A. SNOW AND RAIN LOADS LOCATION: TORONTO (CITY HALL)

ROOFS OF ABOVE GRADE STRUCTURES HAVE BEEN DESIGNED FOR SNOW AND RAIN LOADS IN ACCORDANCE WITH THE FOLLOWING CRITERIA:

- GROUND SNOW LOAD = 0.90 kPa WITH ASSOCIATED RAIN LOAD OF 0.4 kPa. MODIFIED IN ACCORDANCE WITH THE NBC STRUCTURAL COMMENTARY. ICE LOADS ARE INCLUDED IN SNOW LOAD CALCULATION. Is, Ss, Cs, Cw, Ce, Ca, Ss FACTORS ARE TAKEN AS PER OBC 2024 PART 4 CLAUSE 4.1.6. IMPORTANCE CATEGORY: NORMAL
- RAIN LOAD BASED ON THE USE OF CONTROLLED FLOW ROOF DRAINS SATISFYING THE REQUIREMENTS OF THE PLUMBING CODE OF CANADA, RESULTING IN A LOAD OF 1.0 kPa OBC 2024 PART 4 CLAUSE 4.1.6.4 IS CHECKED FOR RAIN LOAD CALCULATION.

B. WIND LOAD

ABOVE GRADE STRUCTURES WILL UTILIZE THE FOLLOWING PARAMETERS ACCORDANCE WITH THE OBC 2024:

- 1 IN 50 YEAR WIND PRESSURE = 0.44 kPa
- Ce, Cg AS PER NBC STRUCTURAL COMMENTARY
- Cp = 2.0
- Cs TAKEN AS CATEGORY 3
- IMPORTANCE FACTOR Iw BASED ON TABLE 4.1.7.1 OF 2024 ONTARIO BUILDING CODE. "NORMAL" CATEGORY ROOF ELEMENTS AND THEIR CONNECTIONS WILL BE DESIGNED FOR UPWARD SUCTION DUE TO WIND PRESSURES.

C. SEISMIC LOAD

- FOR PROPOSED ADDITION AND UPGRADES, SEISMIC ANALYSIS IS NOT REQUIRED FOR THE BUILDING.
- SFRS TYPE:
a) CONVENTIONAL CONSTRUCTION OF MASONRY SHEAR WALLS.

CITY OF TORONTO



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H	ADDENDUM 3	2026-01-12

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PROJECT TITLE
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SERVICES FLEET
MAINTENANCE STATION -
UPGRADE WORKS

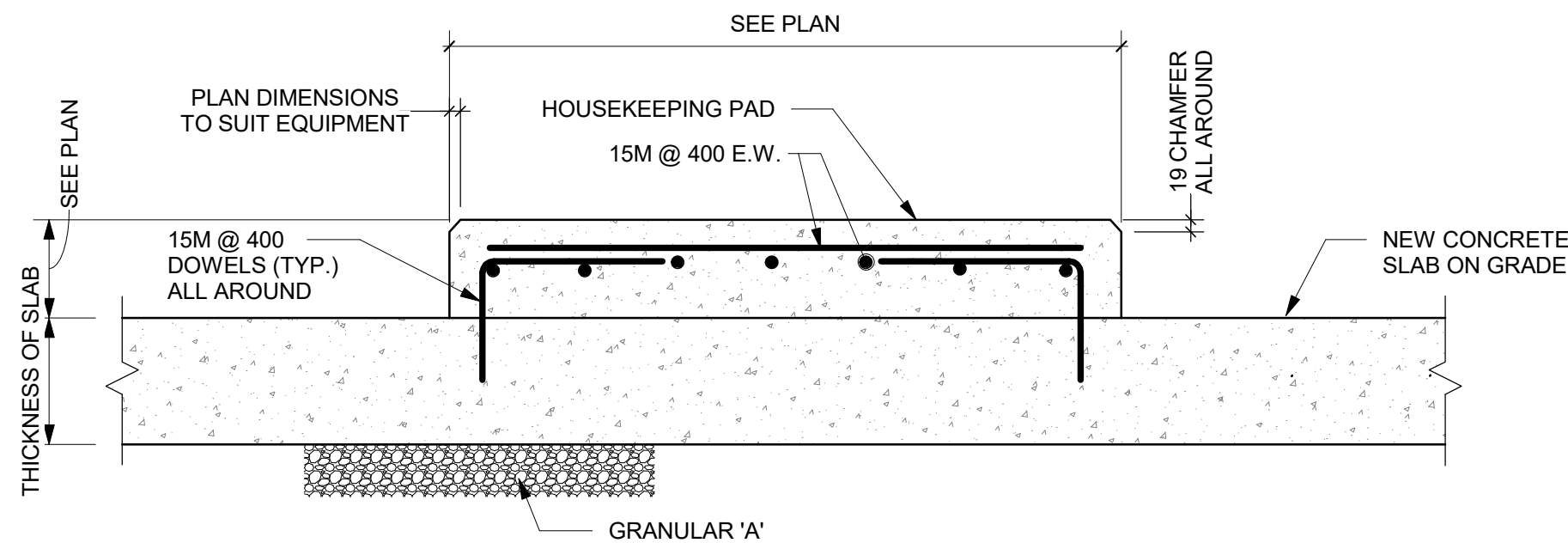
PROJECT ADDRESS

KING STREET YARD -
BUILDING NO. 8 & 9
1116 KING STREET WEST

PROJECT NO: 30276606	
DRAWN BY: D. DU	CHECKED BY: R. CHHATRASHAL
PROJECT MGR: N. LAYOUN	APPROVED BY: M. SHEININ

SHEET TITLE
GENERAL NOTES

SHEET NUMBER	ISSUE
S010	H

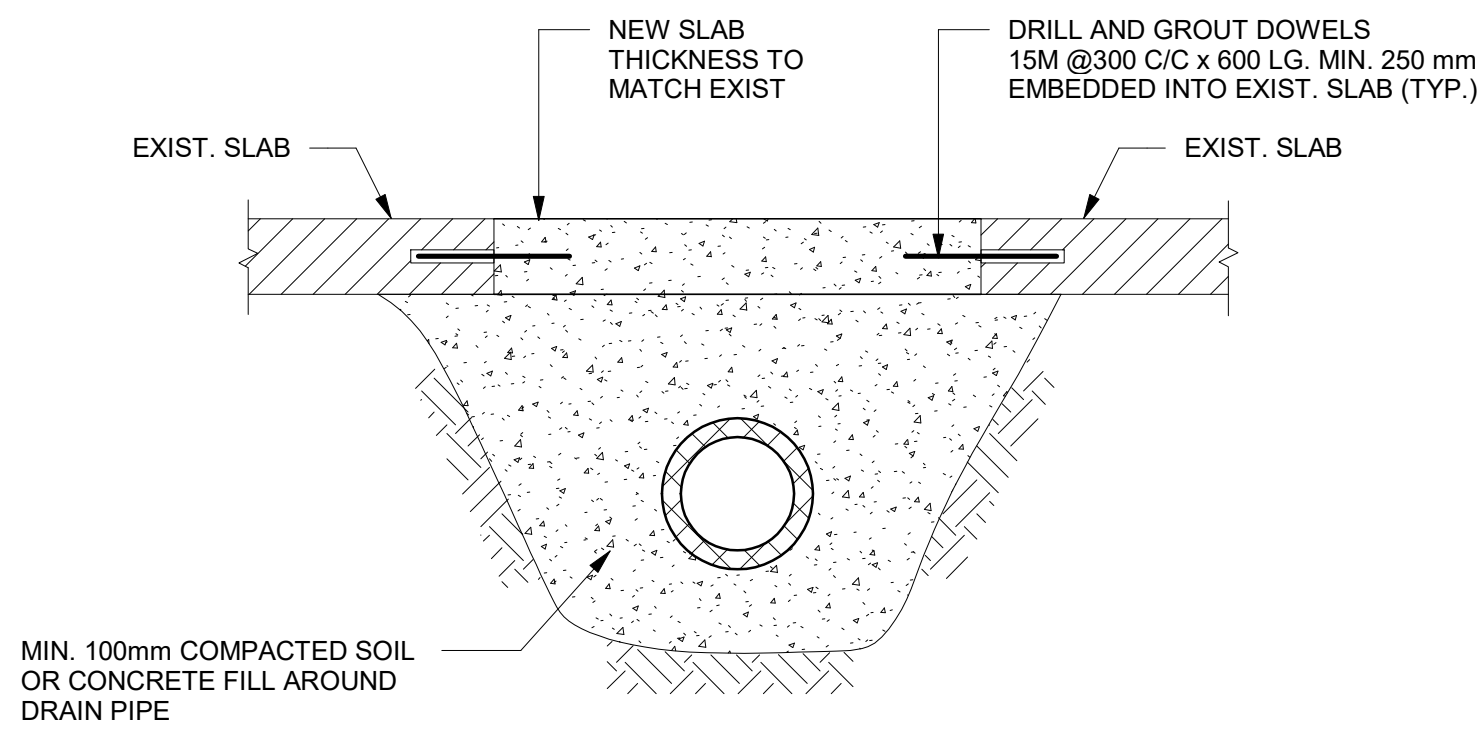


NOTE:

1. MINIMUM 28-DAY COMPRESSIVE STRENGTH OF HOUSEKEEPING PAD CONCRETE SHALL MEET REQUIREMENTS OF MANUFACTURER OF SUPPORTED EQUIPMENT AND THE SEISMIC RESTRAINT DESIGN ENGINEER FOR THE SUPPORTED EQUIPMENT, BUT SHALL NOT BE LESS THAN 30 MPa. INTERIOR HOUSEKEEPING PAD CONCRETE SHALL BE EXPOSURE CLASS C-4, WITH NO AIR ENTRAINMENT.

1 TYPICAL EQUIPMENT PAD ON SLAB

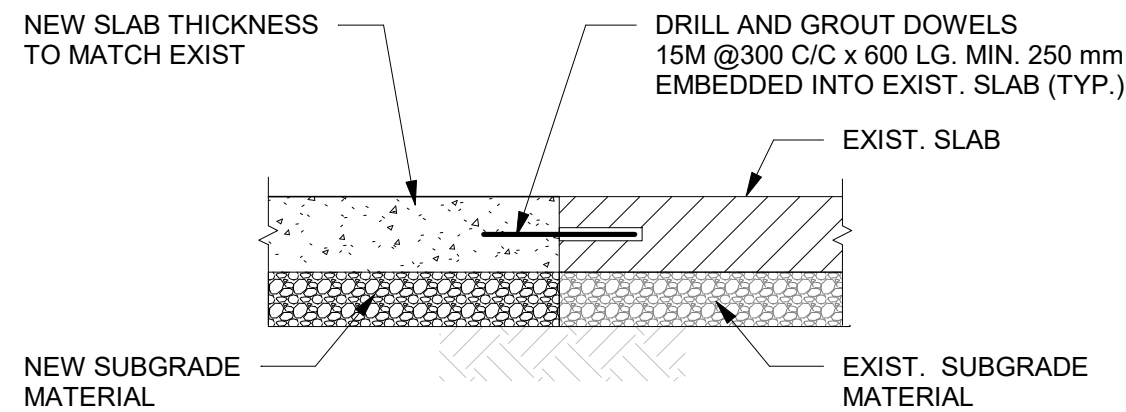
Scale: 1 : 10



TYPICAL DETAIL OF COMPACTED SOIL OR CONCRETE FILL AROUND PIPE BELOW SLAB ON GRADE (IF REQUIRED)

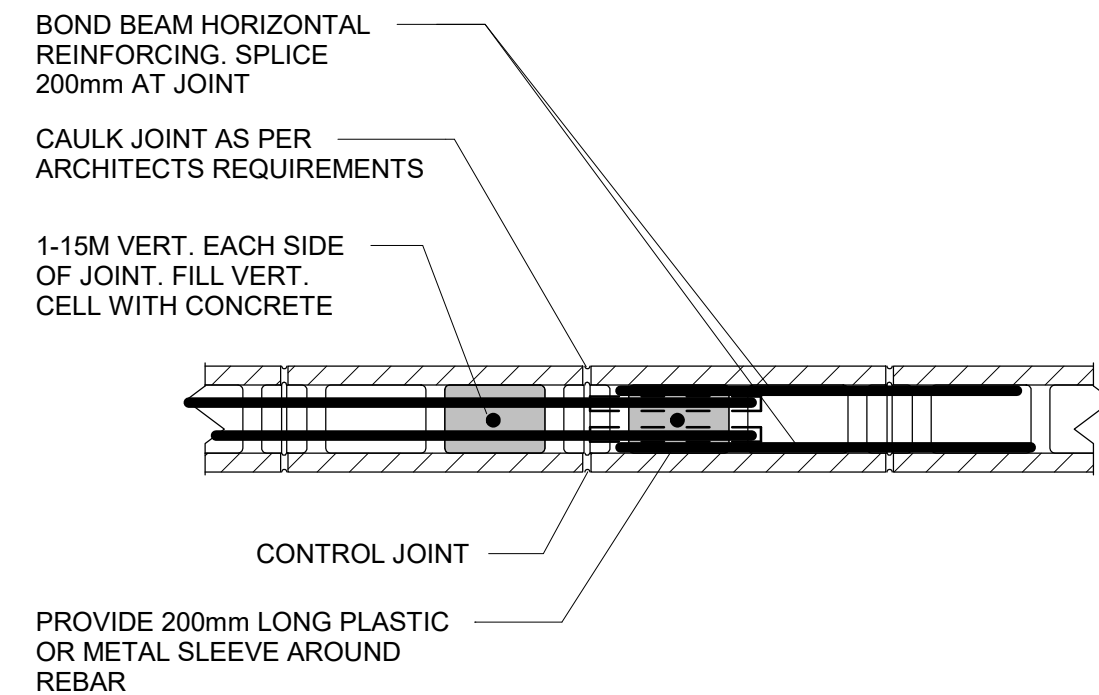
4 TYPICAL PIPE TRENCH DETAIL UNDER EXISTING SLAB ON GRADE

Scale: NTS



5 CONSTRUCTION JOINT WITH EXISTING SLAB

Scale: NTS

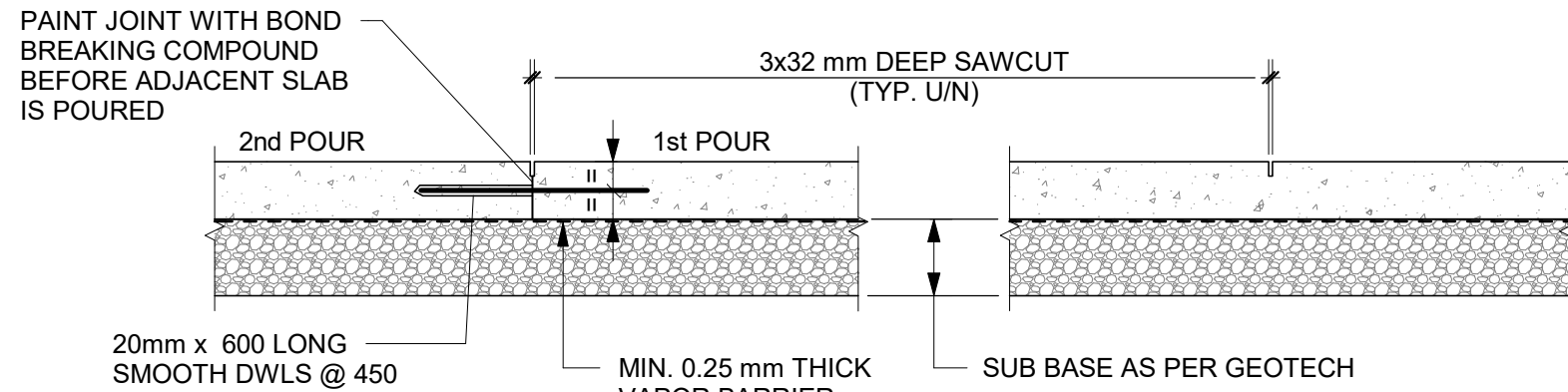


6 TYP. MASONRY WALL CONTROL JOINT

Scale: 1 : 10

NOTES:

1. PLAN SECTION TAKEN THROUGH BOND BEAM.
2. DISCONTINUE HORIZONTAL LADDER REINFORCEMENT EVERY SECOND LAYER.
3. SPACE CONTROL JOINTS AT 6000mm O/C MAXIMUM UNLESS NOTED OTHERWISE ON DRAWINGS.
4. CONTROL JOINTS SHALL NOT BE LOACTED UNDER O.W./S.J. OR STEEL BEAMS.

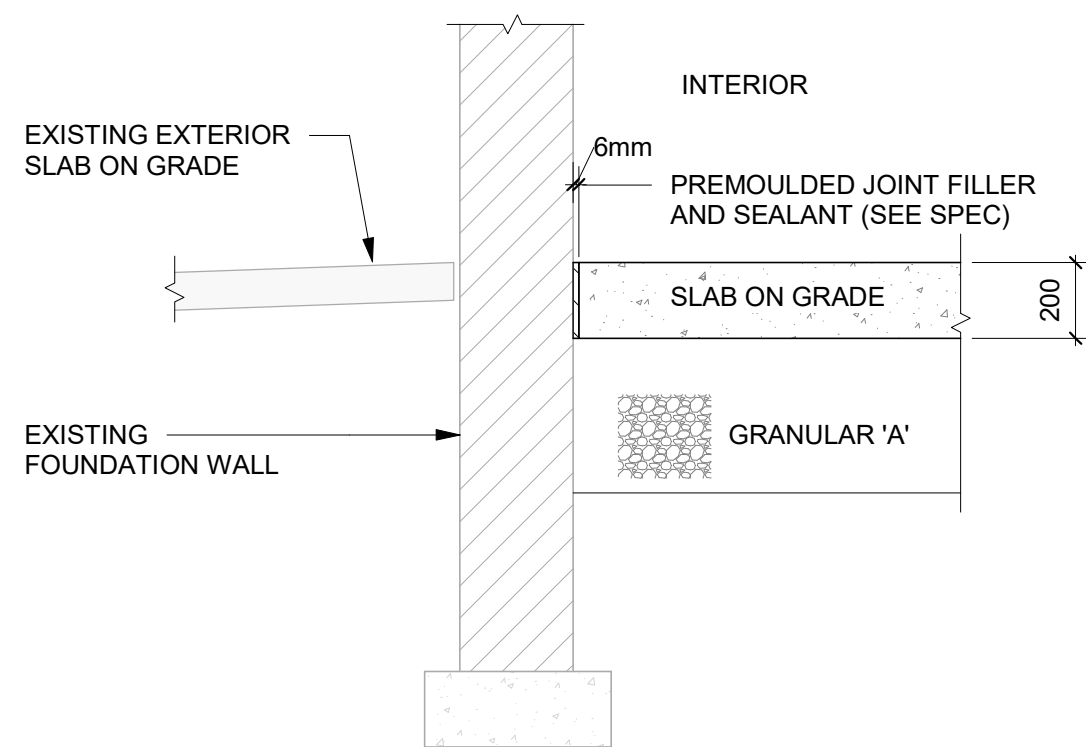


CONSTRUCTION JOINT

CONTROL JOINT

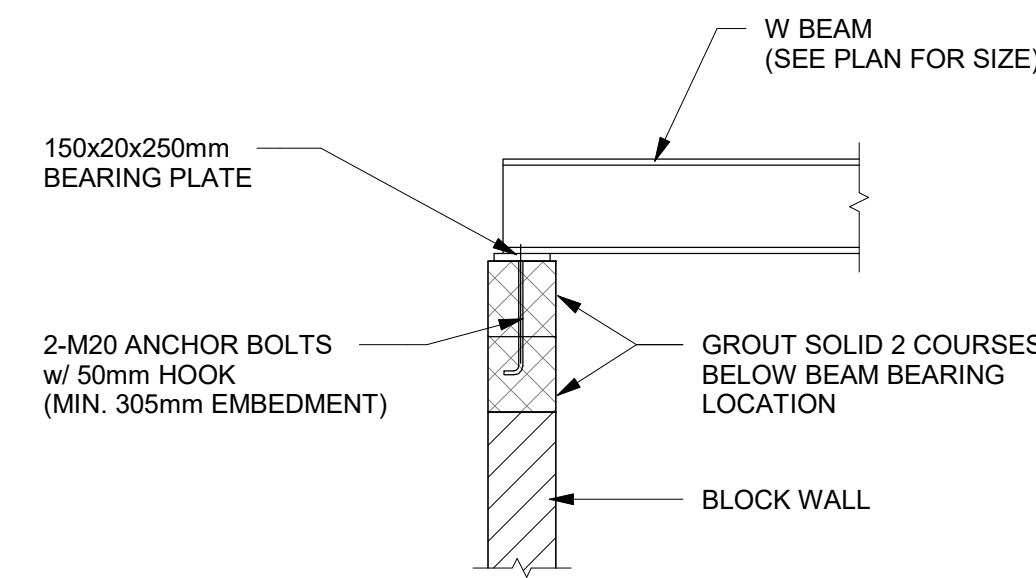
2 NEW SLAB ON GRADE

Scale: 1 : 20



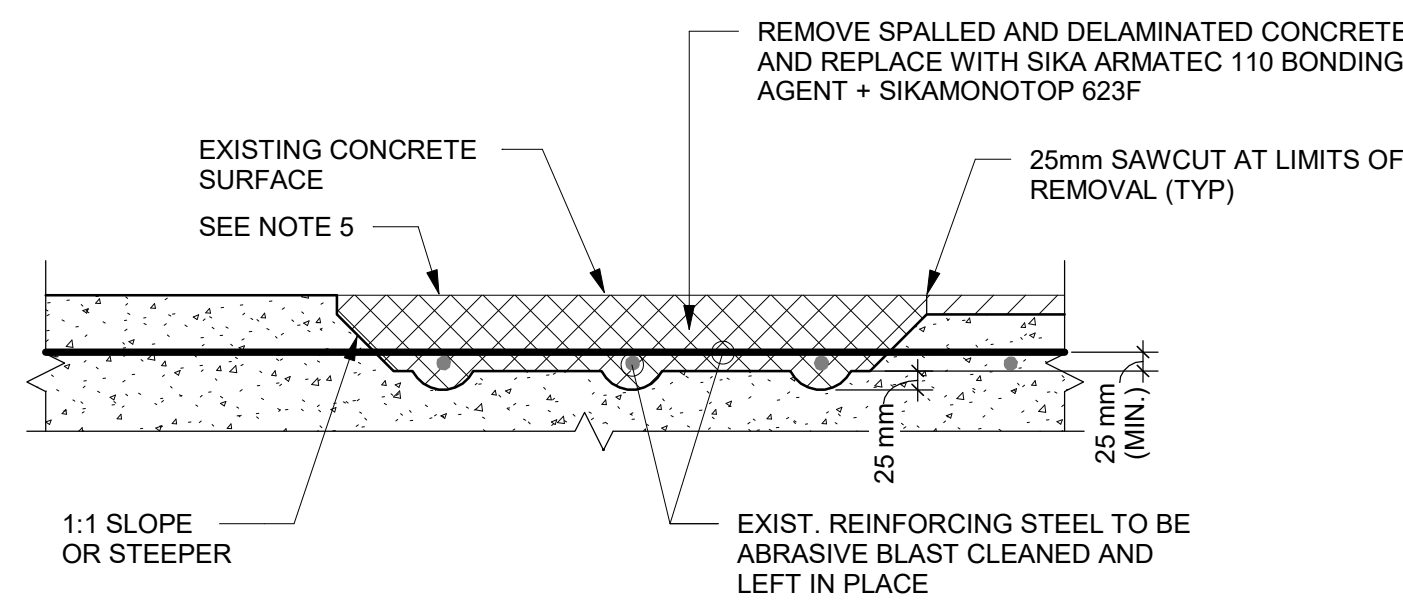
7 TYPICAL PERIMETER WALL ISOLATION

Scale: NTS



8 TYPICAL BEAM BEARING DETAIL AT MASONRY WALL

Scale: 1 : 20

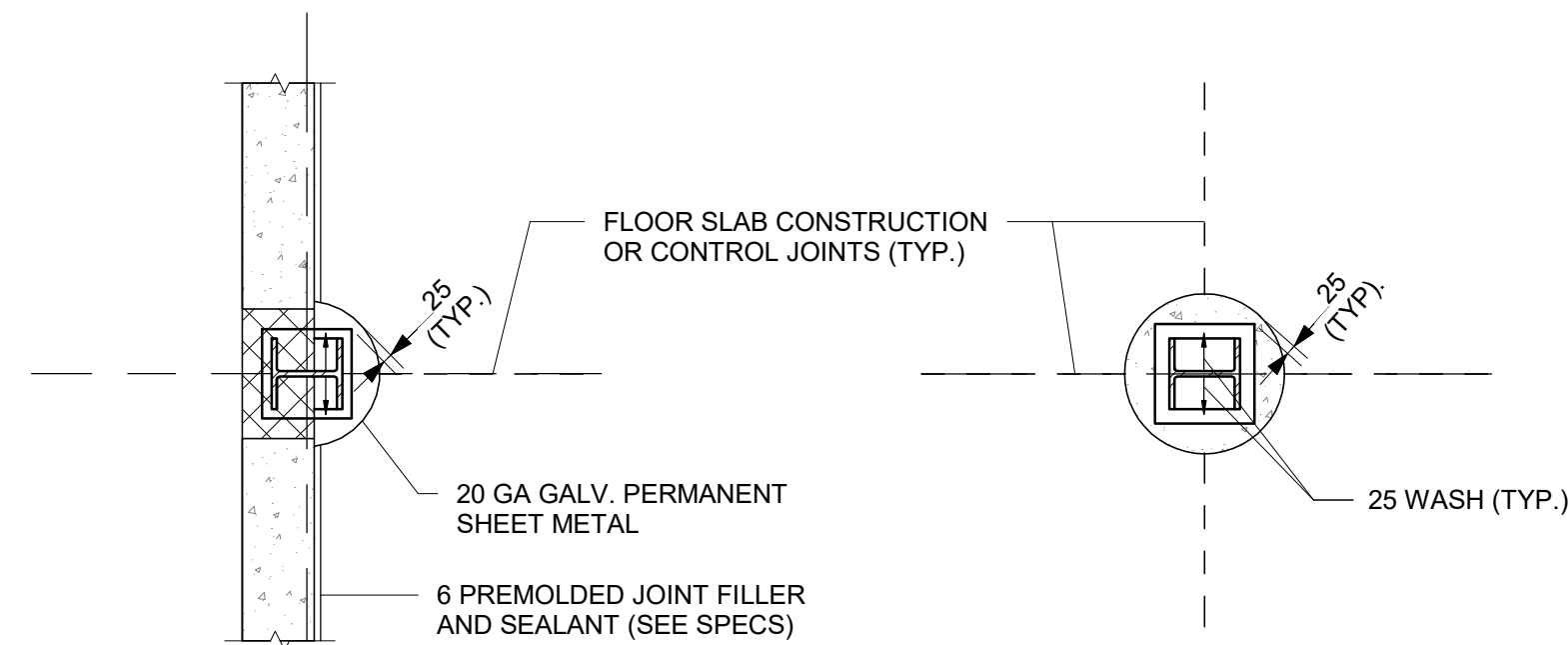


NOTES:

1. CRACK REPAIR WITH EPOXY INJECTION PER SPEC SECTION 03 64 23. BASE QUANTITY FOR BIDDING PURPOSES: 15m.
2. TOP OF SLAB SURFACE REPAIR TO BE PER DETAIL SHOWN. BASE QUANTITY FOR BIDDING PURPOSES: 12m². ASSUME THICKNESS OF REPAIR REQUIRED TO BE MAXIMUM 100mm. NOTIFY ENGINEER WHEN TOP OF SLAB IS EXPOSED AFTER DEMOLITION OF EXISTING FLOORING, TOPPING AND INSULATION.
3. THE QUANTITIES PROVIDED ON THIS DRAWING ARE ESTIMATES FOR BIDDING PURPOSES. PROVIDE UNIT RATES AS REQUIRED BY THE CONTRACT IN ADDITION TO THE BASE QUANTITIES ABOVE. DURING CONSTRUCTION, THE ENGINEER SHALL REVIEW WITH THE CONTRACTOR FOR THE EXACT EXTENT OF AREA WHERE REPAIR IS REQUIRED, TO DETERMINE ACTUAL QUANTITIES.

9 REPAIR DETAIL - TOP OF SLAB SURFACE

Scale: 1 : 10

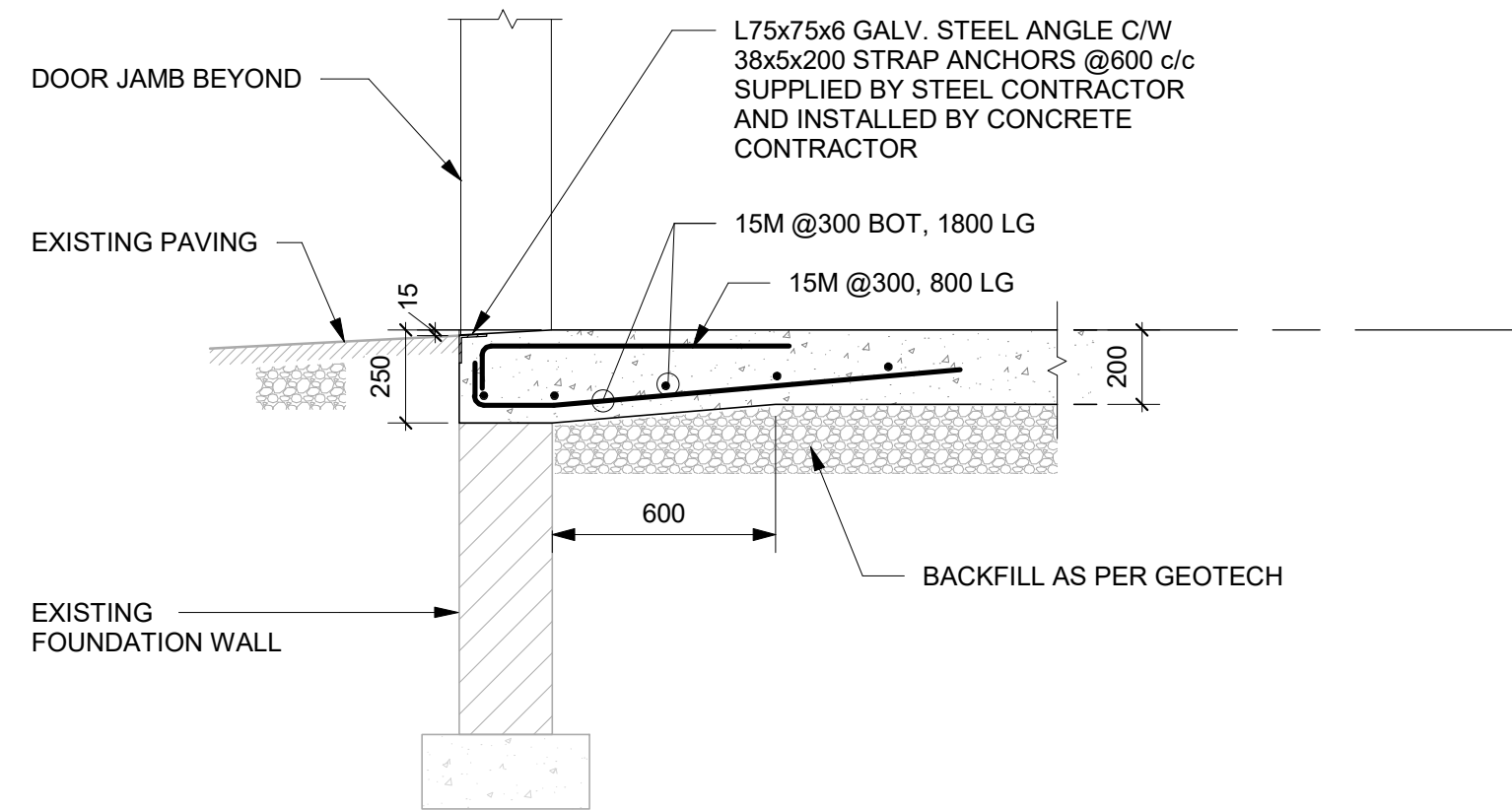


EXTERIOR COLUMN ISOLATION JOINT

INTERIOR COLUMN ISOLATION JOINT

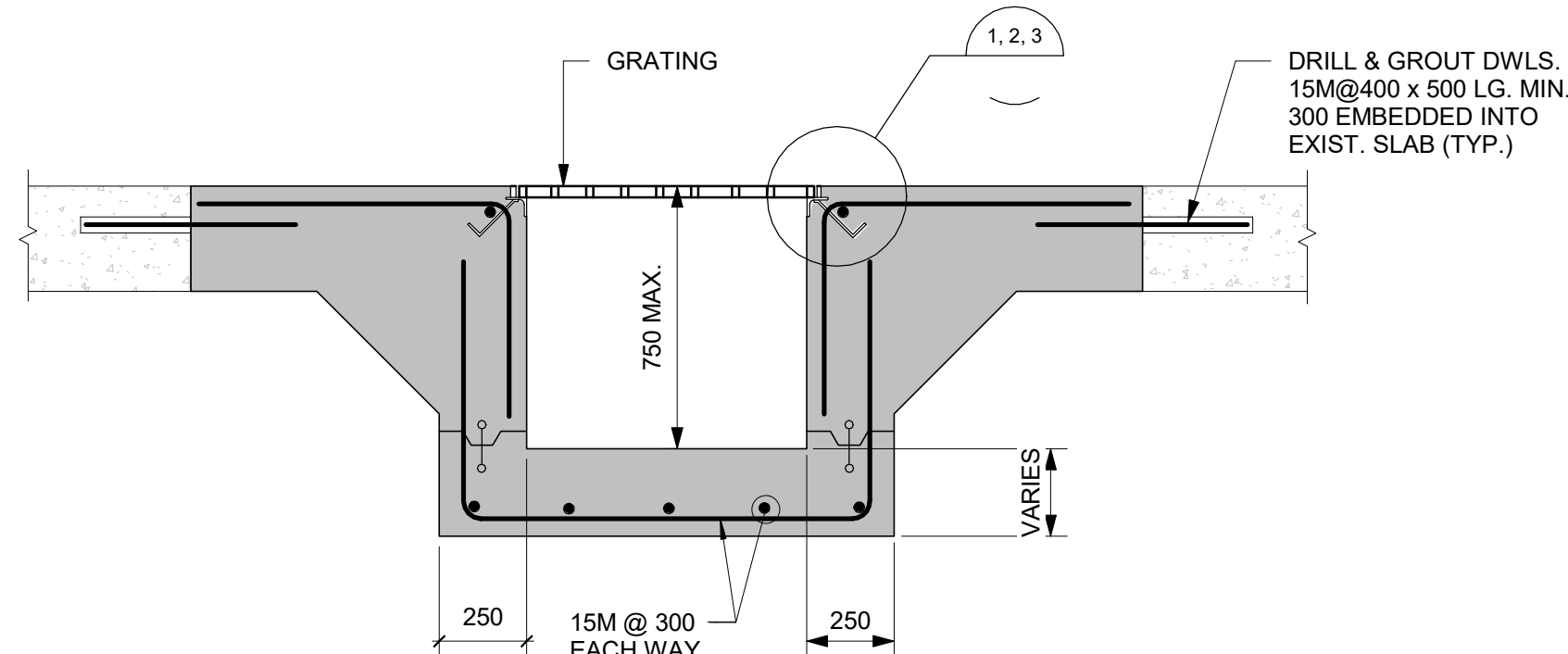
3 COLUMN ISOLATION JOINTS

Scale: 1 : 20

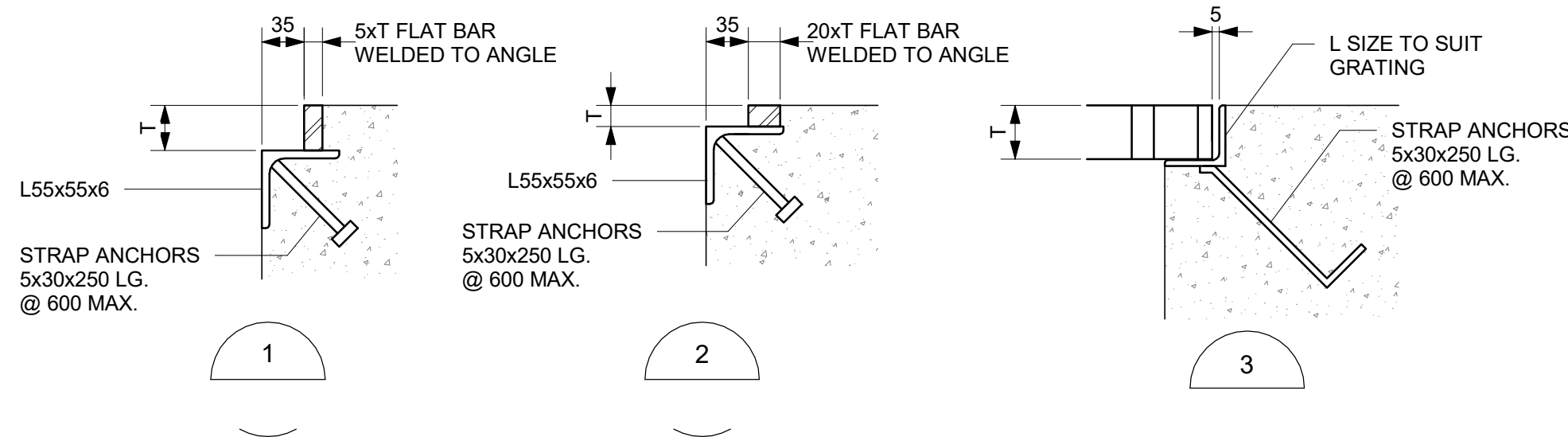


10 TYPICAL DETAIL - DRIVE IN DOOR

Scale: 1 : 20



TRENCH



TRENCH AND PIT COVER FRAMES

SUMP PIT COVER FRAME

NOTE:
TRENCH COVER FRAME DETAIL SIMILAR
PROVIDE ANCHORS AT 600 MAX. SPACING

"T" = THICKNESS OF
GRATING COVER

11 TRENCH AND PIT COVER FRAMES

Scale: 1 : 20

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PROJECT TITLE
**TORONTO PARAMEDIC
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MAINTENANCE STATION -
UPGRADE WORKS**

PROJECT ADDRESS

**KING STREET YARD -
BUILDING NO. 8 & 9
1116 KING STREET WEST**

PROJECT NO:
30276606

DRAWN BY:

D. DU

CHECKED BY:

R. CHHATRASHAL

PROJECT MGR:

N. LAYOUN

APPROVED BY:

M. SHEININ

SHEET TITLE

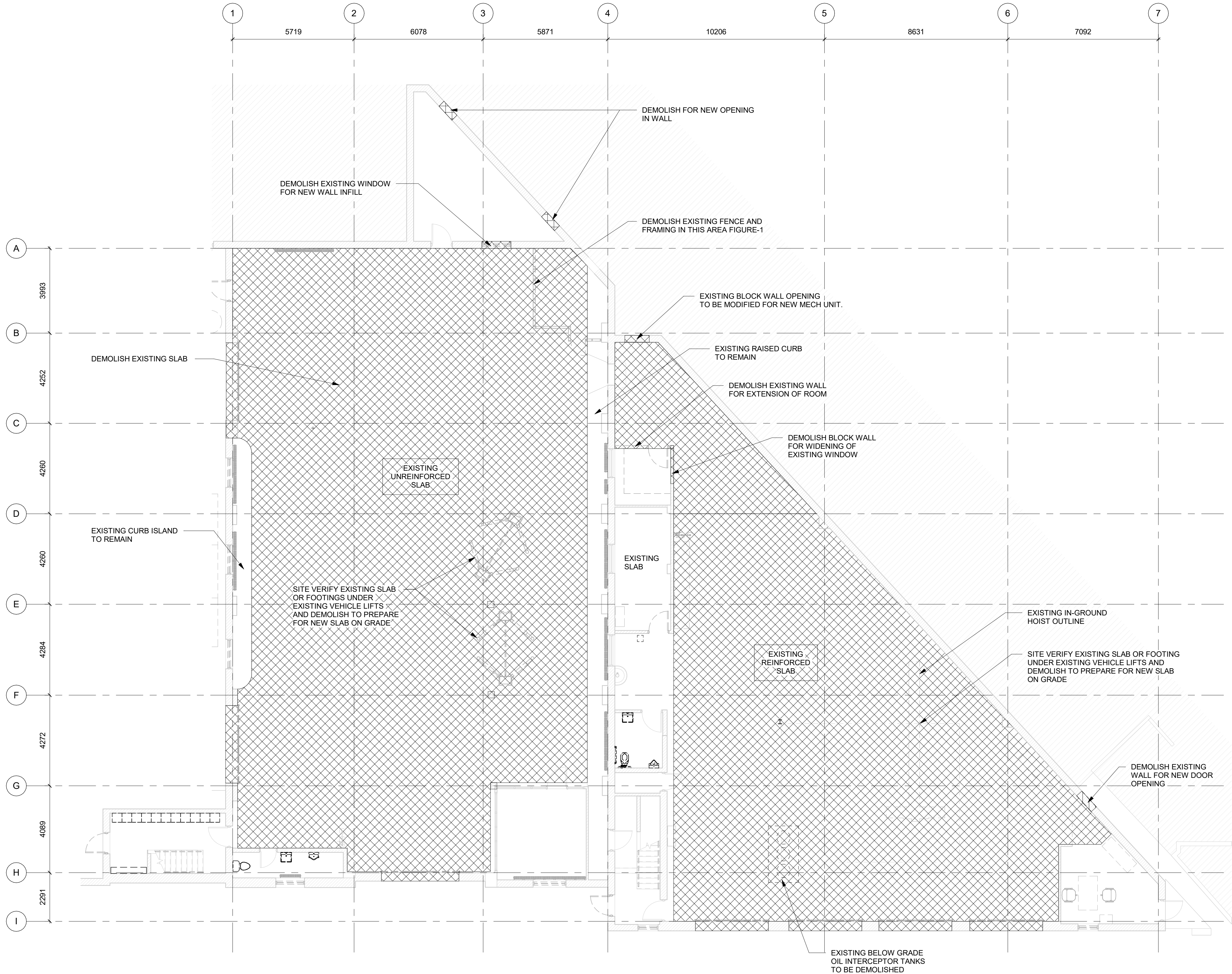
TYPICAL DETAILS 1

SHEET NUMBER

S020

ISSUE

G

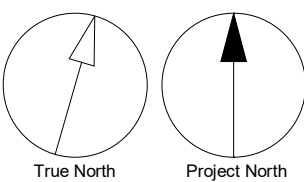
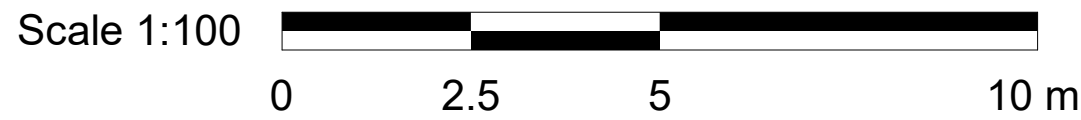


LEGEND:

DEMOLISH EXISTING SLAB TO PREPARE NEW SLAB ON GRADE

- DEMOLITION NOTES:
1. CONTRACTOR SHALL VERIFY ALL THE EXISTING CONDITIONS OF WALLS, SLABS, UTILITIES, EQUIPMENT AND CRACKS BEFORE DEMOLITION TO PREPARE FOR NEW UPGRADES. IF ANY DISCREPANCY NOTED ON SITE, CONTRACTOR SHALL NOTIFY THE ENGINEER ON RECORD FOR REVIEW.
 2. CONTRACTOR SHALL BE RESPONSIBLE TO PROTECT ALL THE EXISTING UTILITIES AND STRUCTURE DURING THE CONSTRUCTION.
 3. CONTRACTOR SHALL BE RESPONSIBLE FOR DESIGN, MEANS AND METHODS TO PROVIDE ANY TEMPORARY SUPPORTS AND SHORING REQUIRED FOR WALLS, PITS AND FOUNDATIONS TO SAFELY EXECUTE THE PROPOSED NEW WORK AT ALL TIMES.
 4. ALL PERIMETER WALLS OF BOTH THE BUILDINGS ARE LOAD BEARING WALLS. THERE IS NOT ENOUGH INFORMATION ABOUT THE BUILDING FOUNDATIONS AND LOAD BEARING WALL ASSEMBLY, THICKNESS & GAP, VENEER AND REINFORCEMENT IF ANY, APPLICABLE IN THESE WALLS AT ANY LOCATION. CONTRACTOR SHALL SITE VERIFY AND PERFORM NECESSARY INVESTIGATION BEFORE DEMOLITION IN THE SELECTED AREAS OF WALL.

1 SLAB ON GRADE PLAN - DEMOLITION
S100 Scale: 1 : 100



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PROJECT ADDRESS

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PROJECT NO:

30276606

DRAWN BY:

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CHECKED BY:

R. CHHATRASHAL

PROJECT MGR:

N. LAYOUN

APPROVED BY:

M. SHEININ

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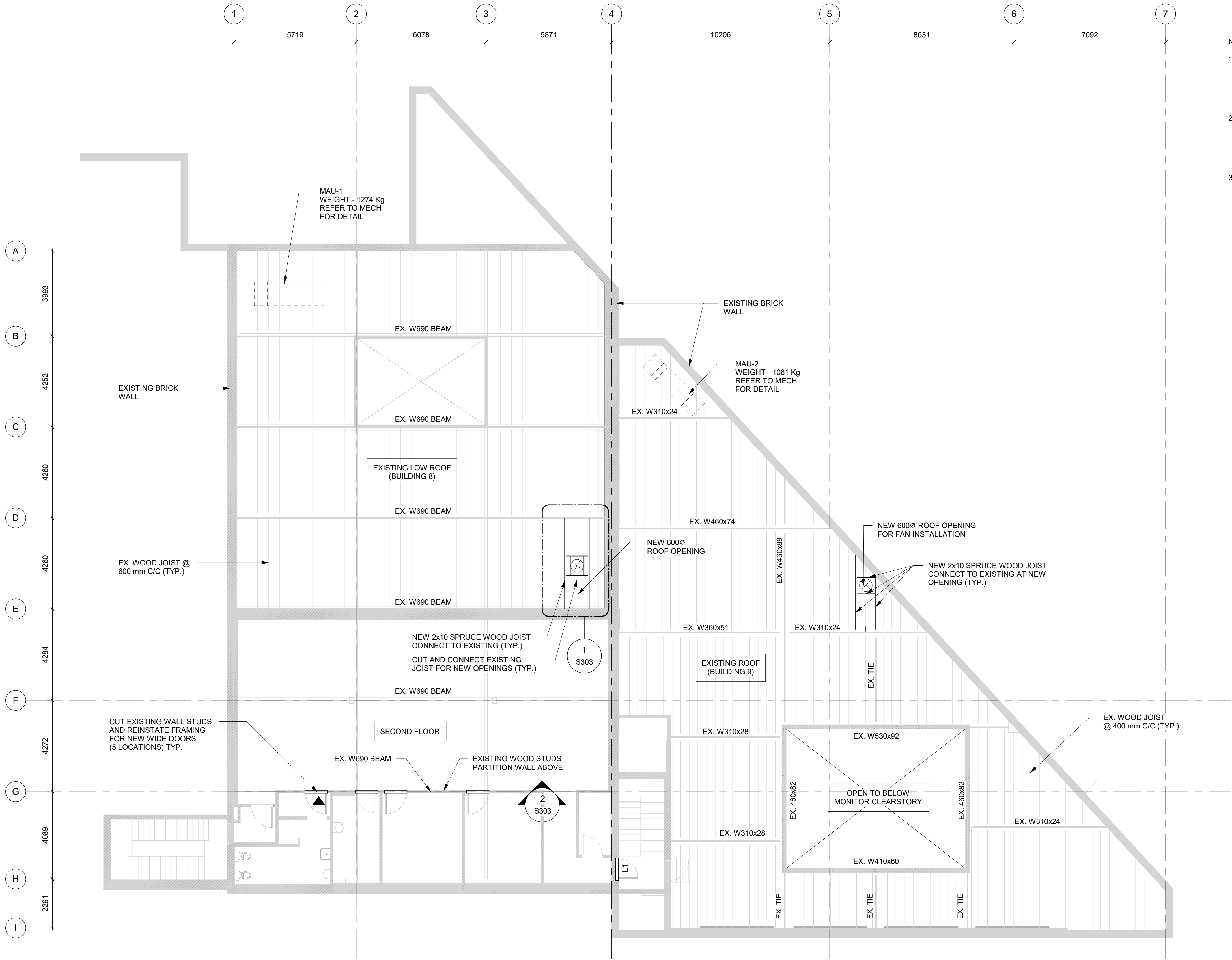
FIRST FLOOR PLAN - DEMOLITION

SHEET NUMBER

S100

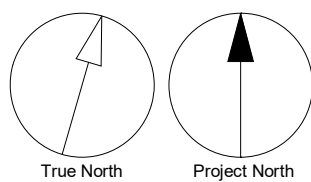
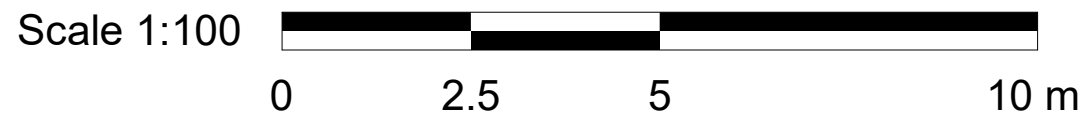
ISSUE

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- NOTES:
- ALL ROOF FRAMING MEMBERS SIZE AND LOCATIONS ARE BASED ON THE EXISTING DRAWINGS PROVIDED BY COT, DATED MAY 1955. CONTRACTOR SHALL VERIFY ALL THE EXISTING CONDITIONS ON SITE BEFORE PROCEEDING FOR DEMOLITION AND FABRICATION. FOR ANY DISCREPANCY NOTED ON SITE, CONTRACTOR WILL INFORM AND CONSULT ARCADIS ENGINEER .
 - CONTRACTOR SHALL COORDINATE WITH MECHANICAL AND ARCH FOR ALL ROOF OPENINGS. ALL NEW WOOD JOISTS AND OPENING FRAMING MEMBER CONNECTIONS DESIGN AND SHOP DRAWINGS SHALL BE DONE BY WOOD SUPPLIER UNLESS NOTED OTHERWISE. CONTRACTOR SHALL SUBMIT SHOP DRAWING STAMPED BY P.ENG. IN THE PROVINCE OF ONTARIO BEFORE FABRICATION.
 - CONTRACTOR SHALL COORDINATE WITH MAKE UP AIR UNIT (MAU) MANUFACTURER FOR SUPPORTS AND CONNECTION WITH WOOD JOISTS AND SUBMIT SHOP DRAWINGS FOR REVIEW.

1 LOWER ROOF PLAN - PROPOSED
S120 Scale: 1 : 100



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PROJECT MGR:
N. LAYOUN

APPROVED BY:
M. SHEININ

SHEET TITLE
**LOWER ROOF PLAN -
PROPOSED**

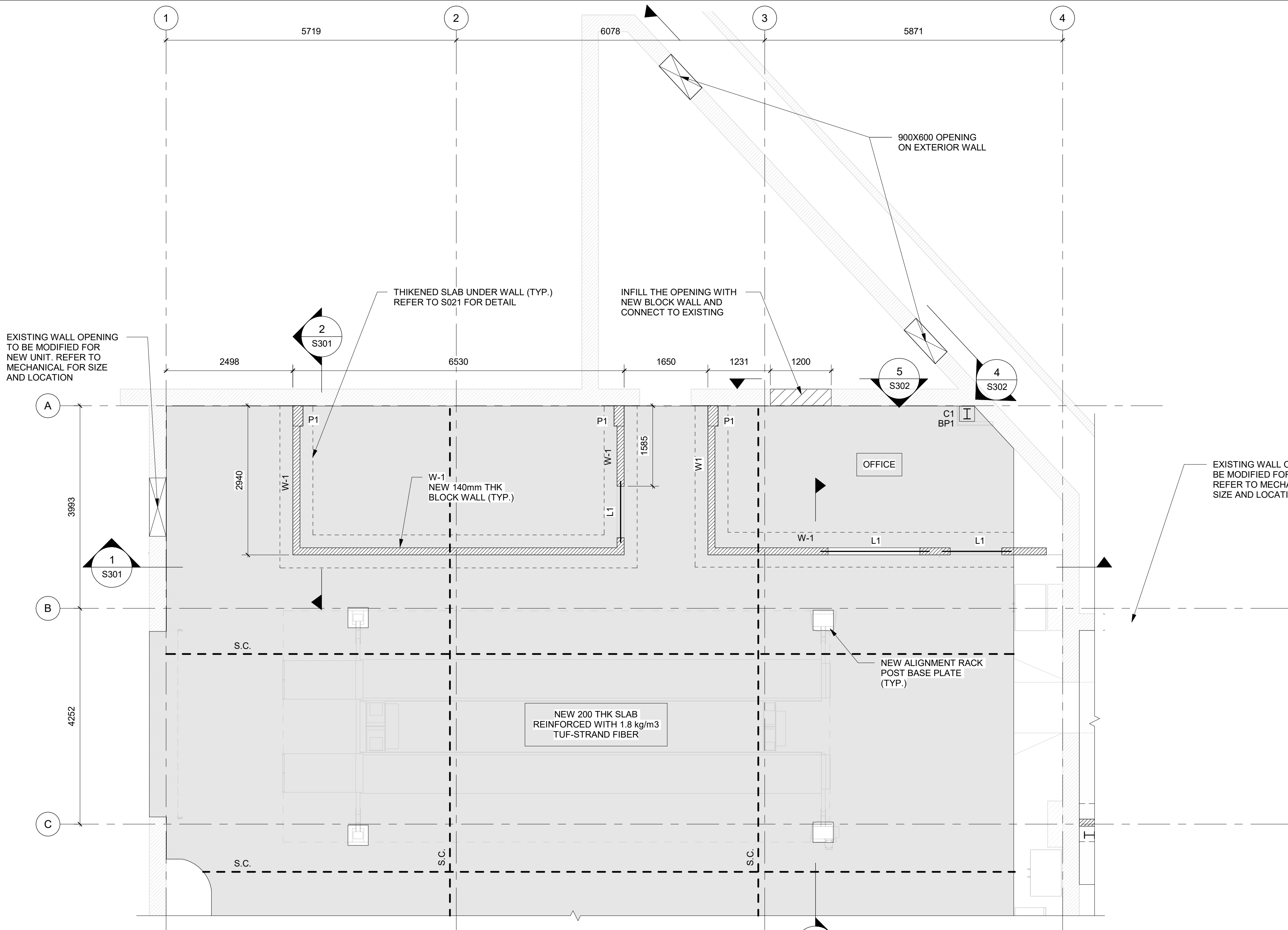
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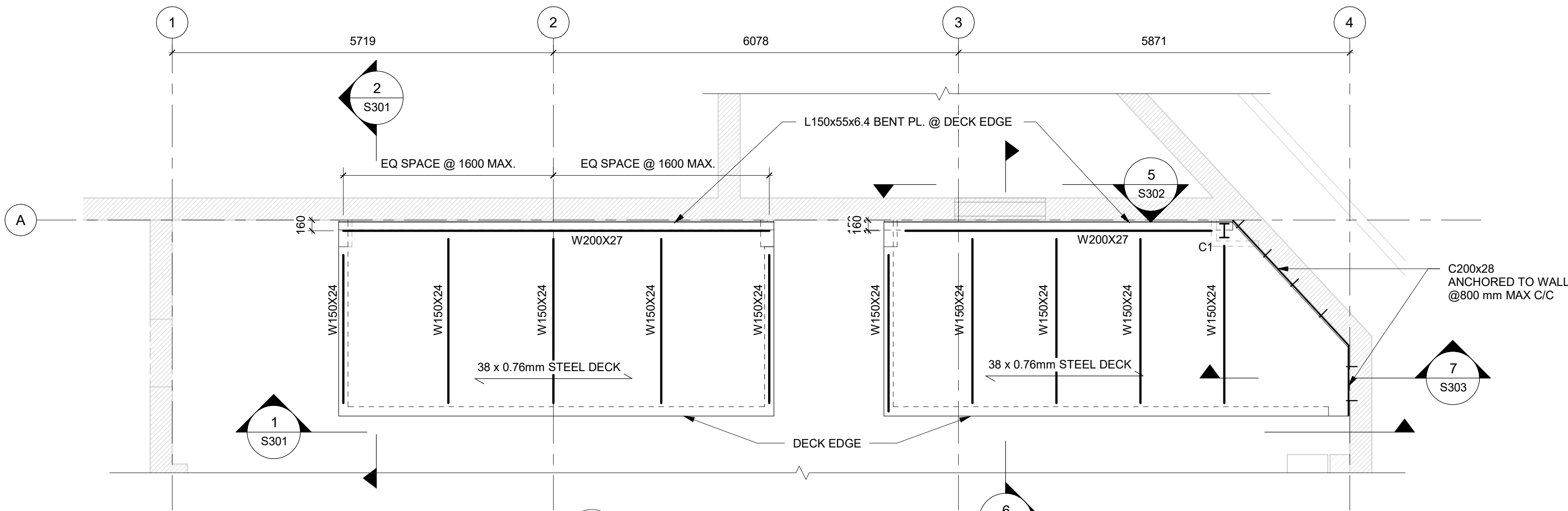
ISSUE

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1 PARTIAL PLAN - FIRST FLOOR
S201 Scale: 1 : 50



4 PARTIAL PLAN - ROOF FRAMING
S201 Scale: 1 : 50

COLUMN SCHEDULE				
MARK	SIZE (mm)	BASE PLATE SIZE (mm)	ANCHOR BOLTS	REMARK
C1	W200x27	BP1 - 250x300x20 THK	4 - 20 mm DIA	SEE 3/S303 FOR DETAIL
		BP2 - 250x300x20 THK	4 - 20 mm DIA	SEE 3/S303 FOR DETAIL

MASONRY PIER SCHEDULE			
MARK	SIZE (mm)	VERTICAL REINFORCEMENT	HORIZONTAL REINFORCEMENT
P1	200x400	2-15M	- TRUSS OR LADDER 4.76mm DIA GALVANIZED WIRE EVERY OTHER COURSE

MASONRY WALL SCHEDULE			
MARK	THICKNESS (mm)	VERTICAL REINFORCEMENT	HORIZONTAL REINFORCEMENT
W-1	140	15M @800 mm	TRUSS OR LADDER 4.76mm DIA GALVANIZED WIRE EVERY OTHER COURSE
W-2	90	--	TRUSS OR LADDER 4.76mm DIA GALVANIZED WIRE EVERY OTHER COURSE

LINTEL SCHEDULE		
MARK	TYPE	REMARK
L1	BLOCK LINTEL	REFER TO DETAIL ON S021
L2	STEEL LINTEL	REFER TO DETAIL ON S021

INTERIOR ROOF FRAMING MEMBER DESIGN LOADS:

DEAD LOAD:
STEEL DECK ----- 0.15 KPa
CEILINGS AND FIXTURES ----- 0.1 KPa

LIVE LOAD: 1.0 KPa

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N. LAYOUN

APPROVED BY:
M. SHEININ

SHEET TITLE
**PARTIAL PLANS -
BUILDING 8**

SHEET NUMBER

S201

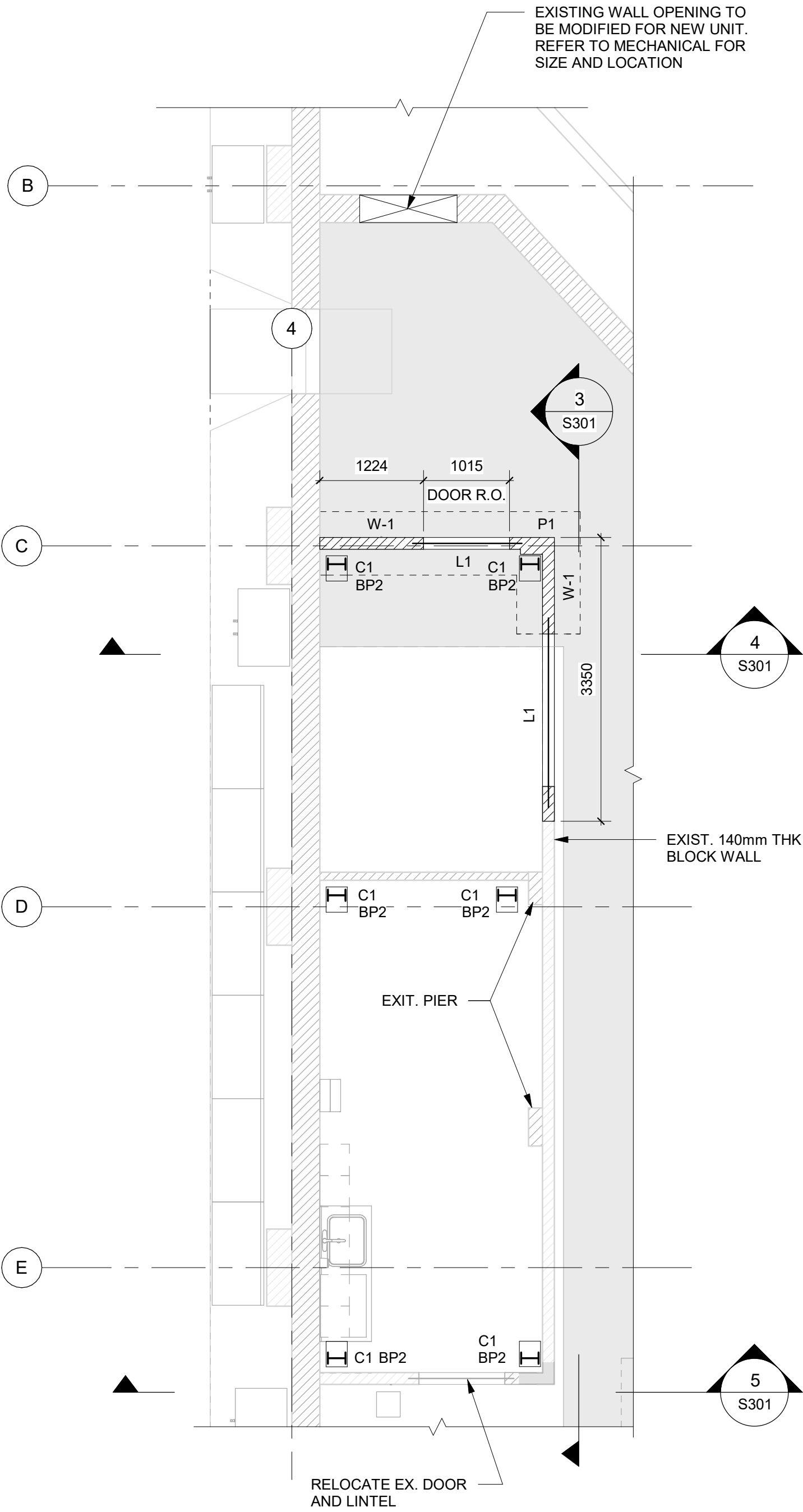
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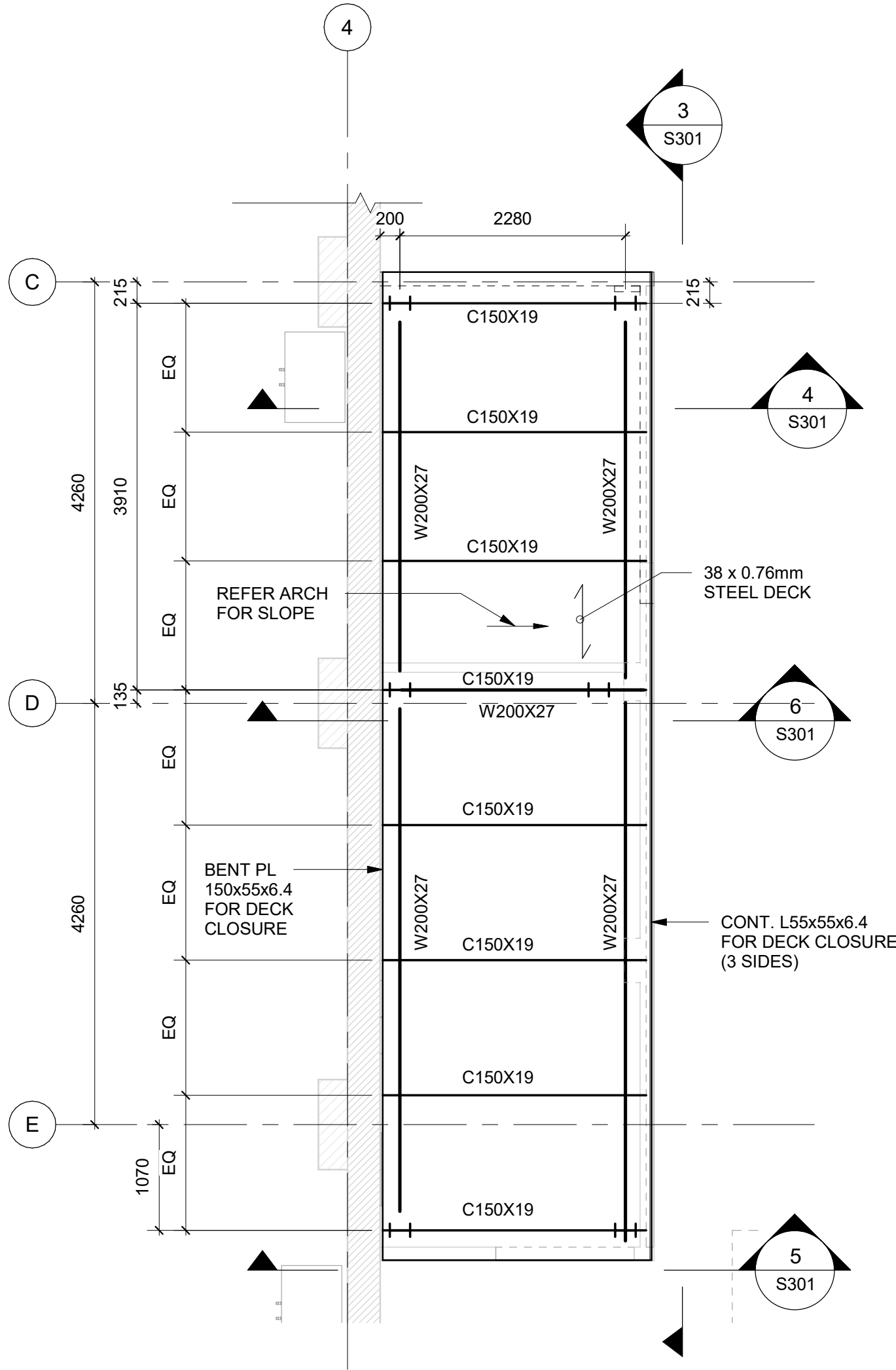
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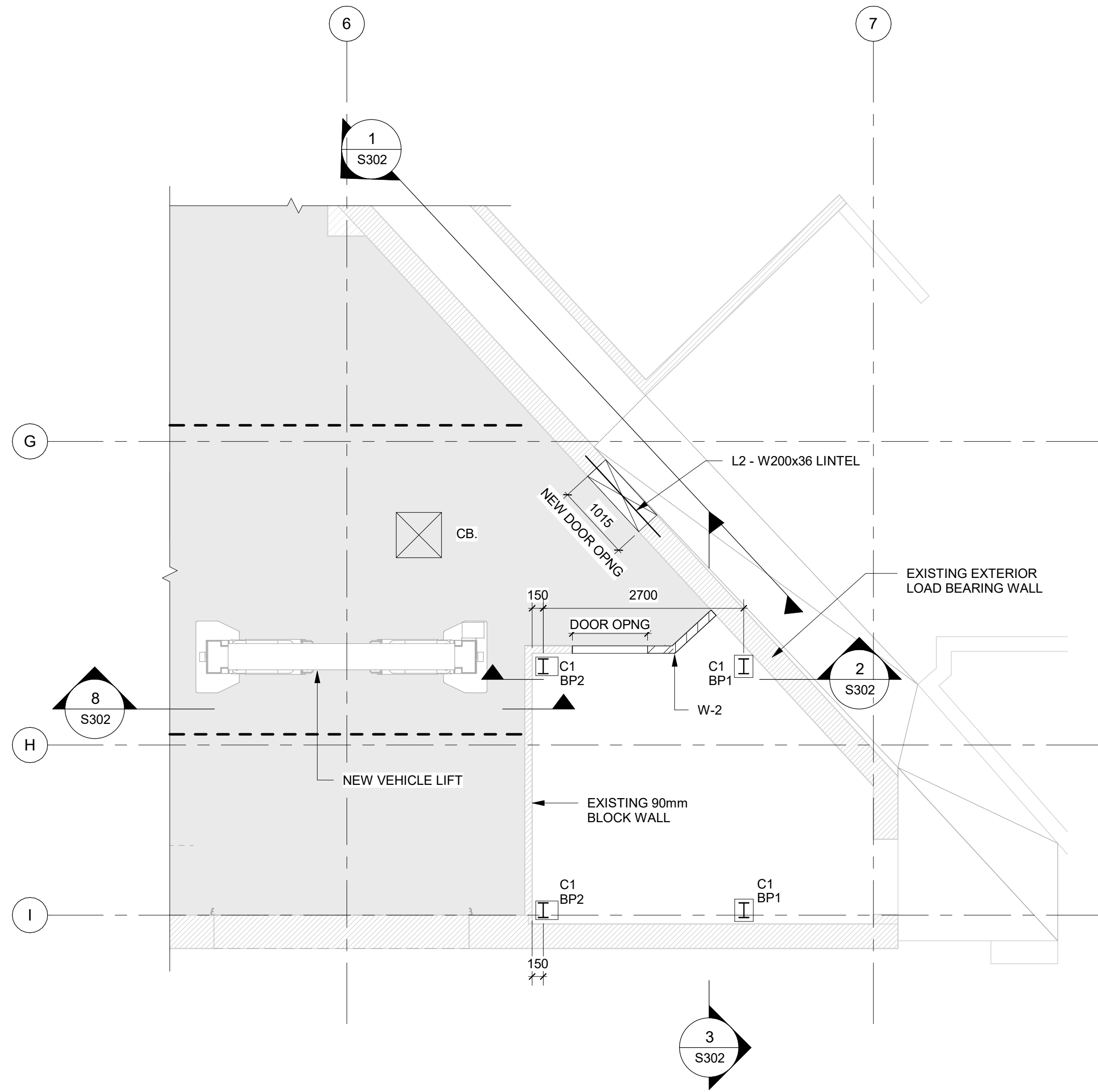
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1"=1m



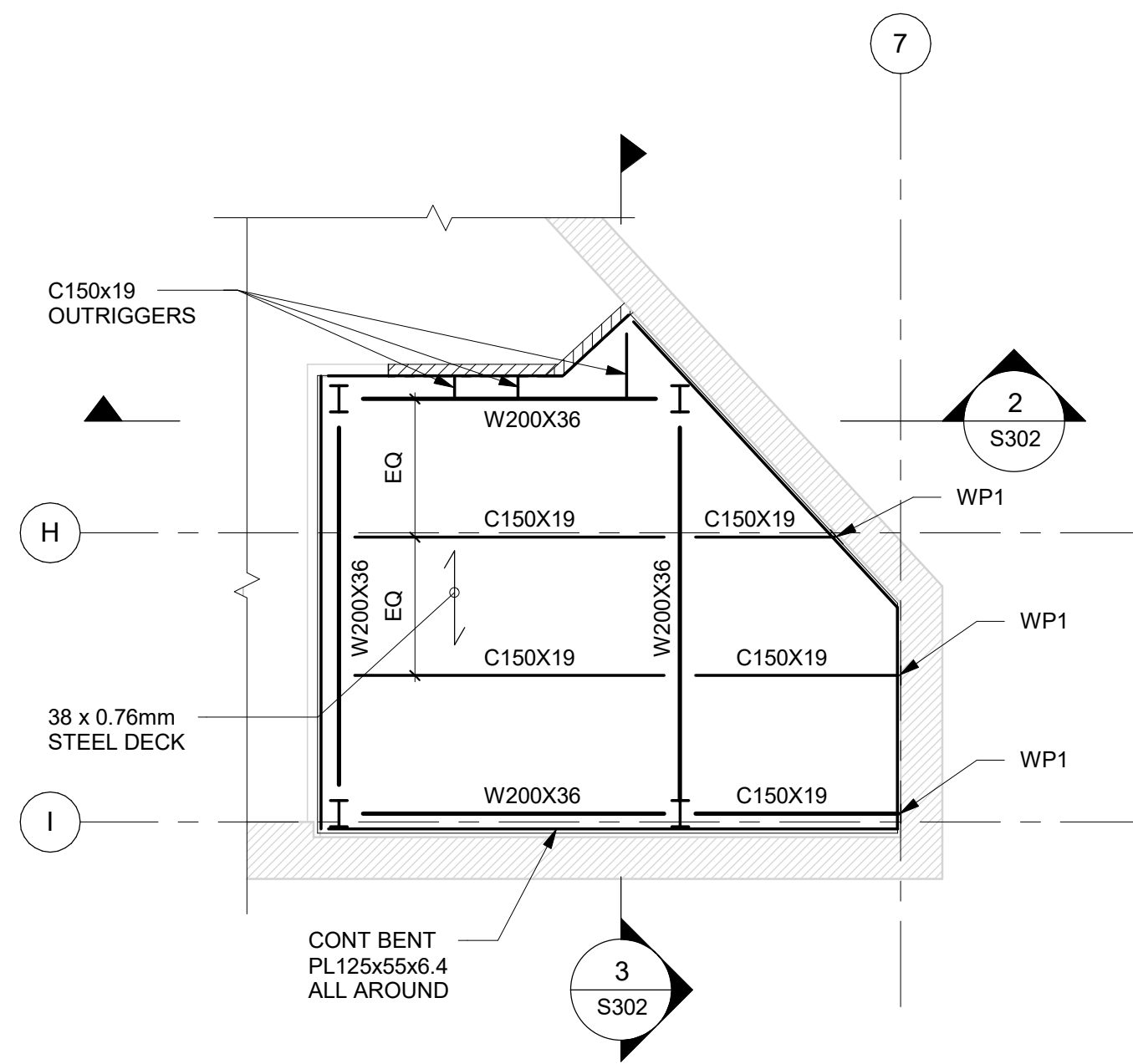
1 PARTIAL PLAN - FIRST FLOOR (OFFICE & LUNCH ROOM)
S202 Scale: 1 : 50



2 PARTIAL PLAN - ROOF FRAMING (OFFICE & LUNCH ROOM)
S202 Scale: 1 : 50



3 PARTIAL PLAN - FIRST FLOOR (STORAGE ROOM)
S202 Scale: 1 : 50



4 PARTIAL PLAN - ROOF FRAMING (STORAGE ROOM)
S202 Scale: 1 : 50

NOTES:
1. REFER TO S201 FOR COLUMN, PIER, WALL AND LINTEL SCHEDULES.

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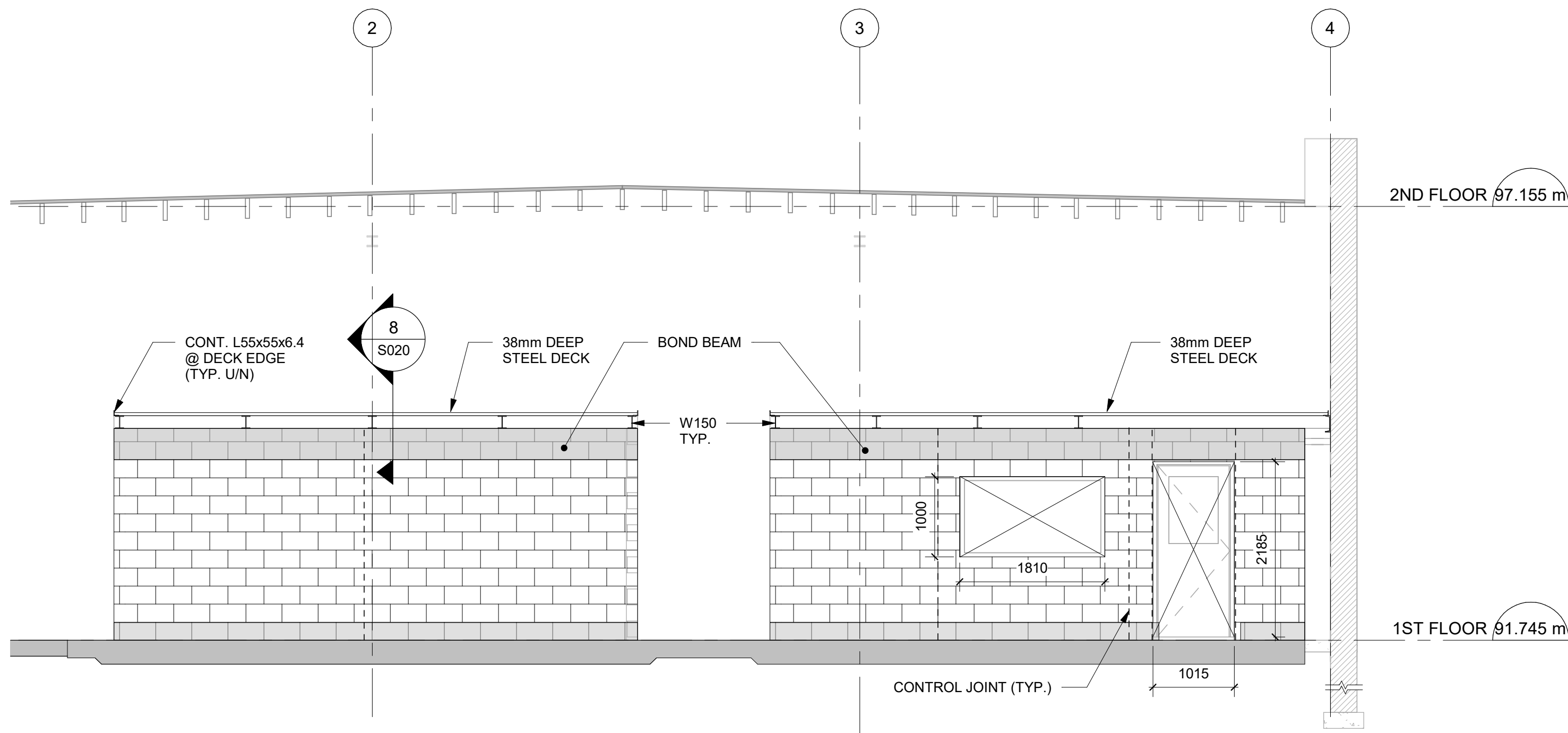
SHEET TITLE
**PARTIAL PLANS -
BUILDING 9**

SHEET NUMBER

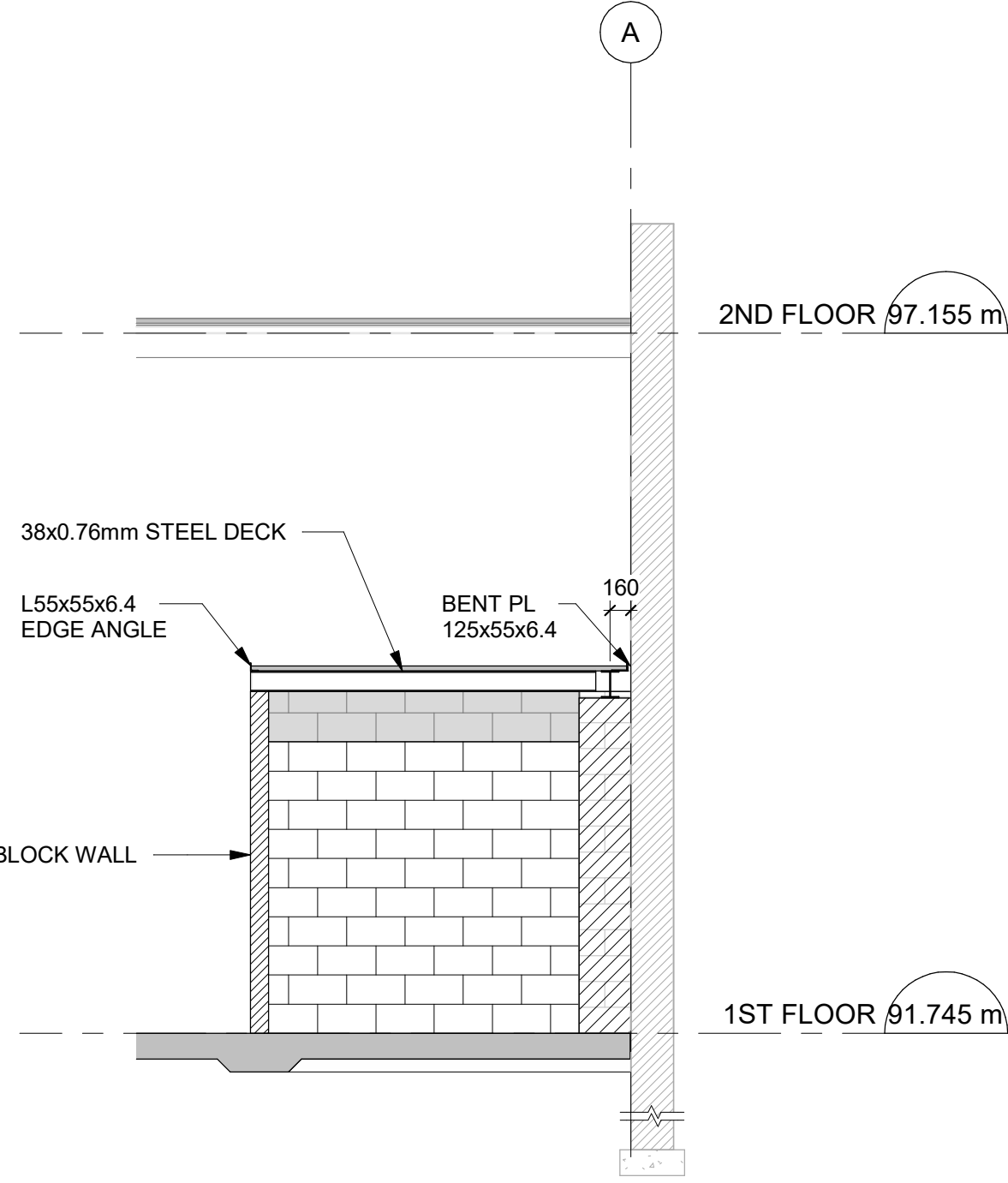
S202

ISSUE

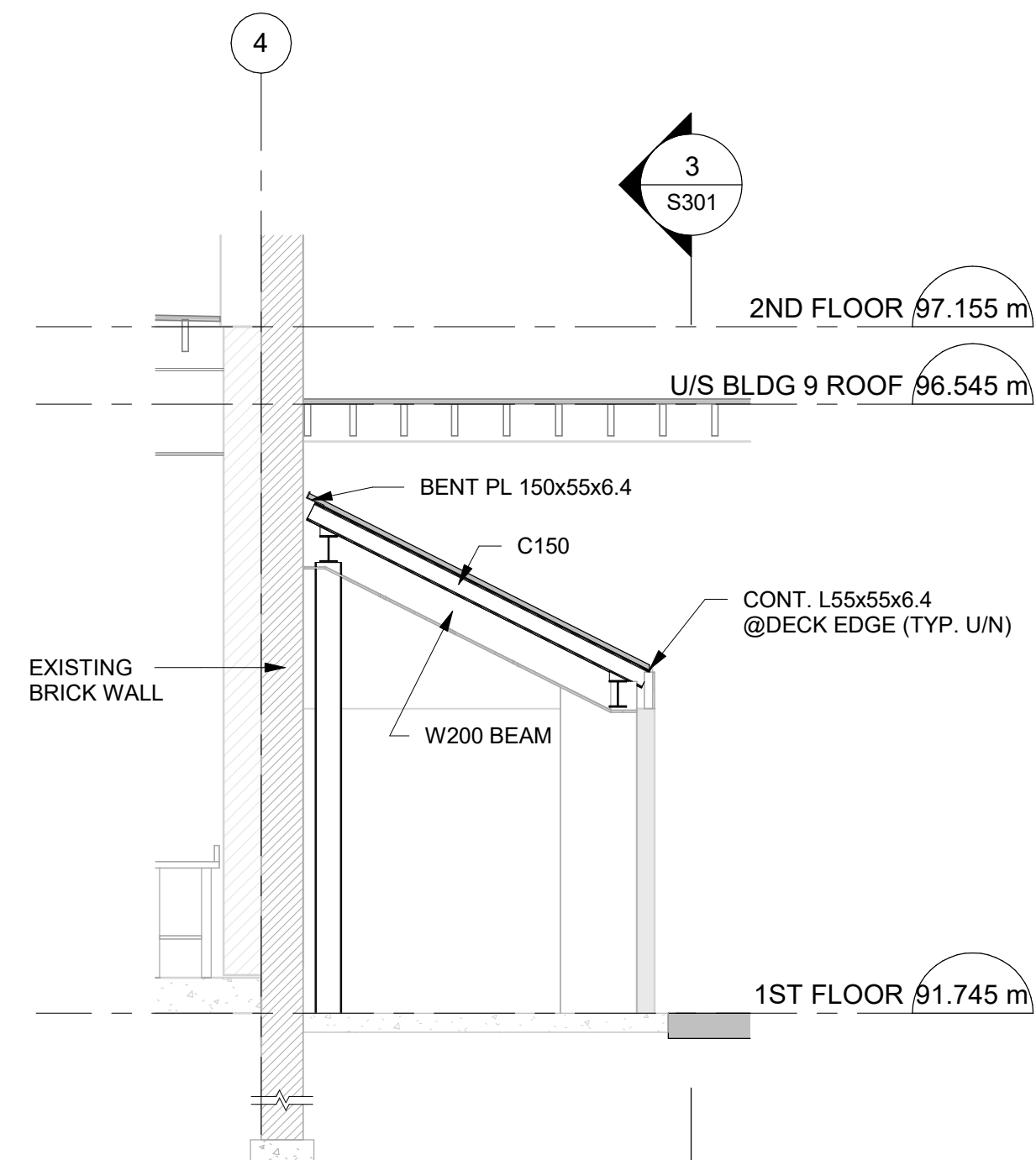
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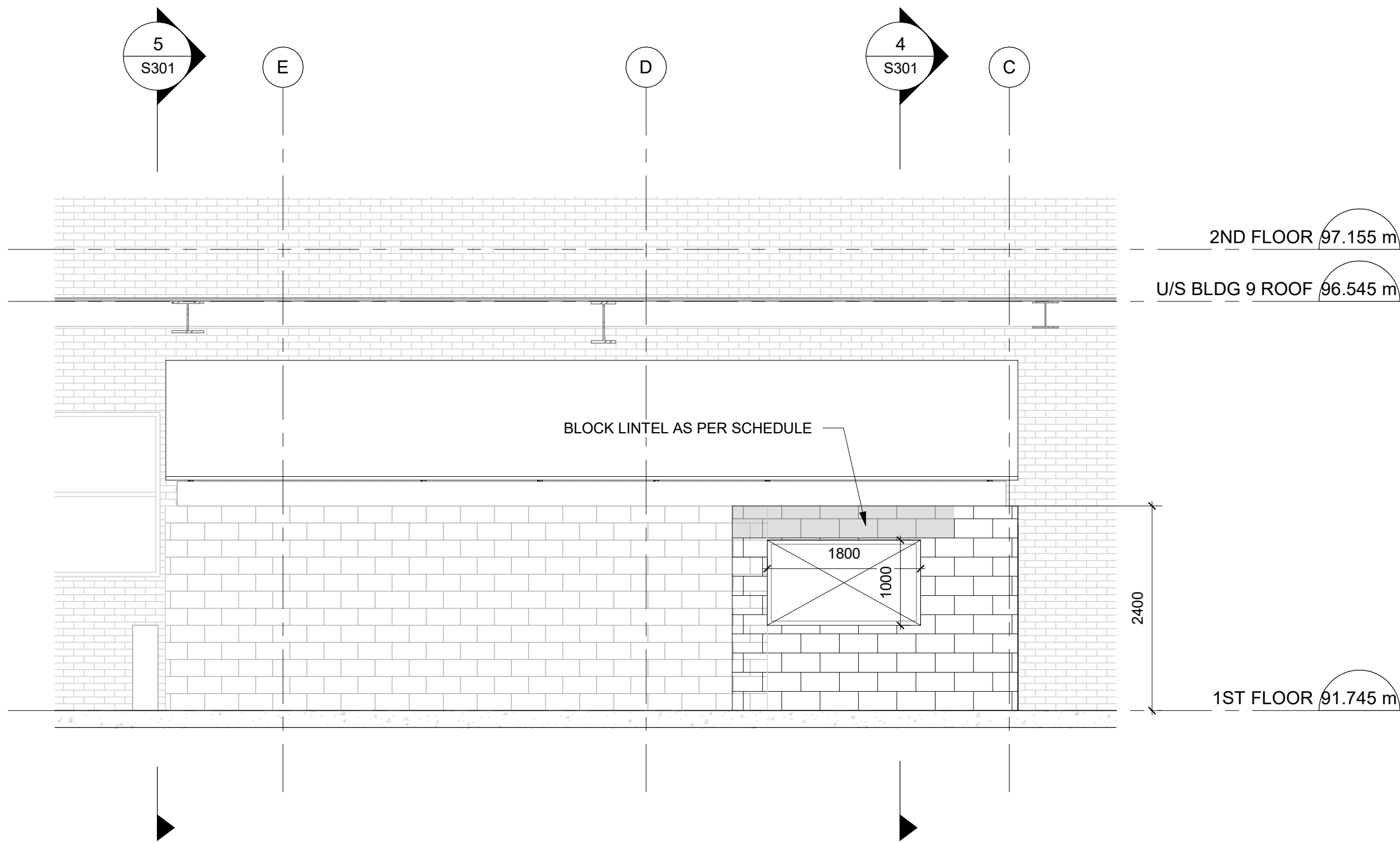
1 SECTION - BUILDING 8
S301 Scale: 1 : 50



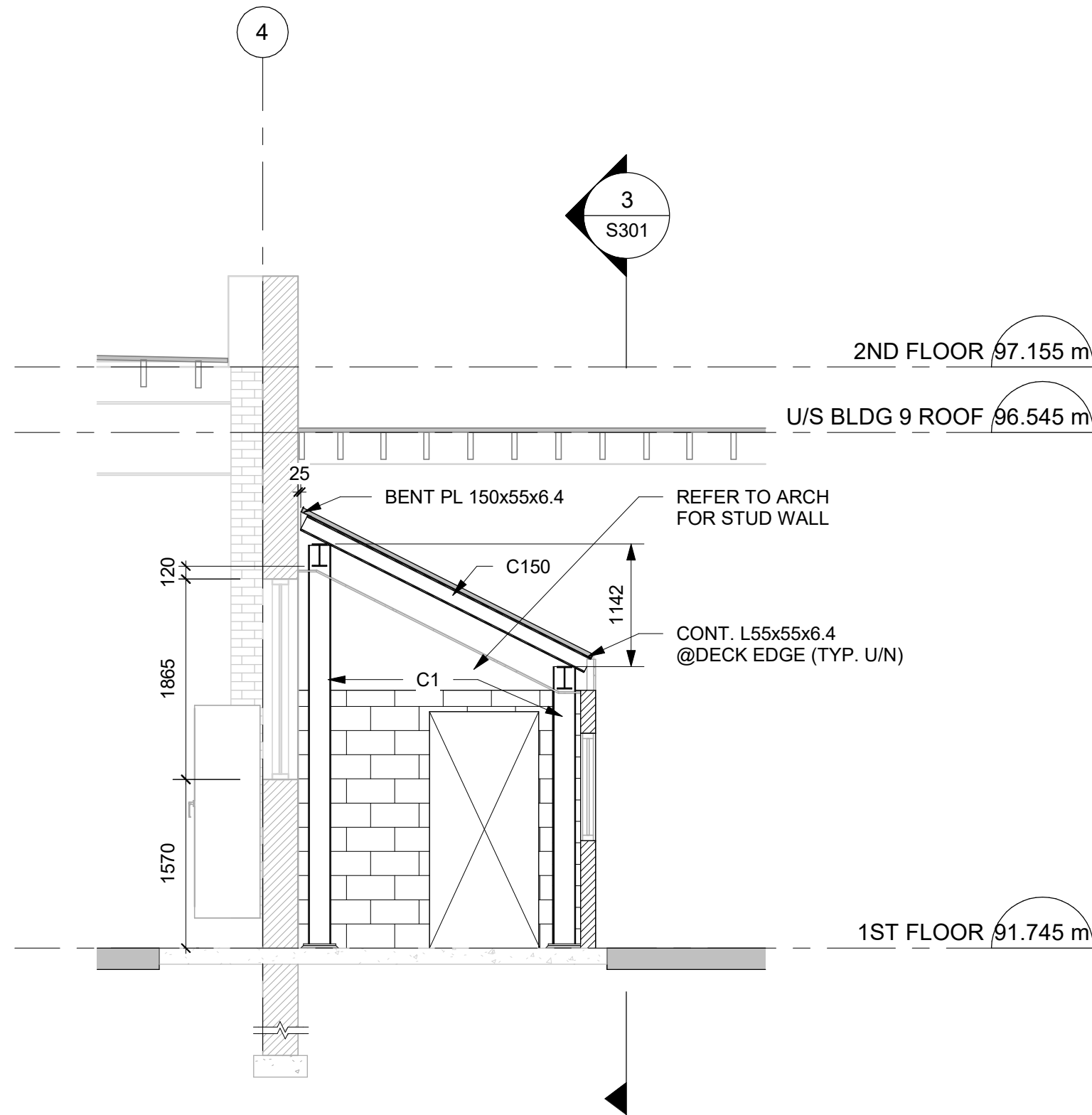
2 SECTION - BUILDING 8
S301 Scale: 1 : 50



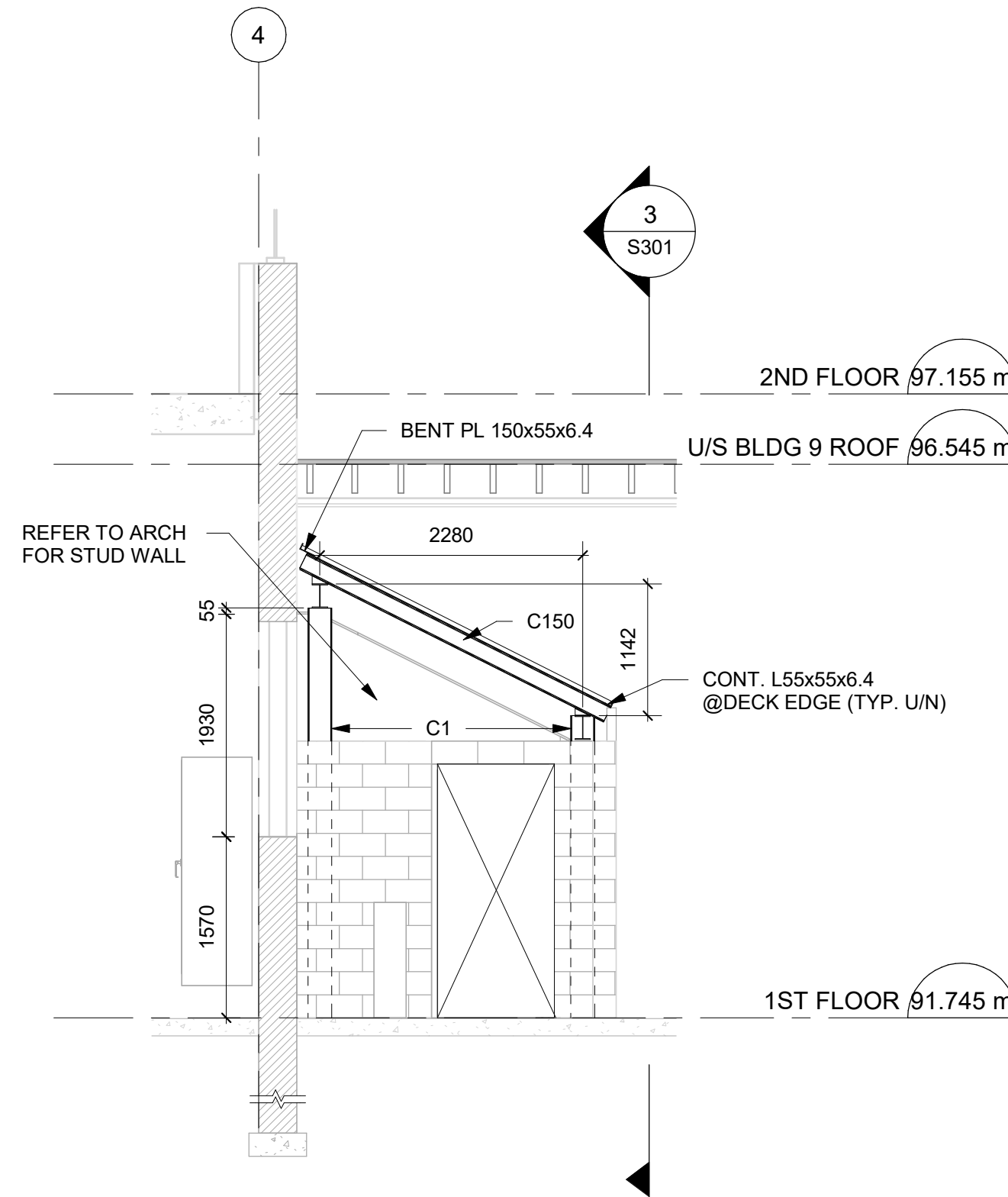
6 SECTION - BUILDING 9
S301 Scale: 1 : 50



3 SECTION - BUILDING 9
S301 Scale: 1 : 50



4 SECTION - BUILDING 9
S301 Scale: 1 : 50



5 SECTION - BUILDING 9
S301 Scale: 1 : 50

ISSUES		
No.	DESCRIPTION	DATE
A	ISSUED FOR 50% REVIEW	2025-10-17
B	ISSUED FOR 75% REVIEW	2025-11-05
C	ISSUED FOR 95% REVIEW	2025-11-28
D	ISSUED FOR 100% REVIEW	2025-12-09
E	ISSUED FOR TENDER	2025-12-09
F	ISSUED FOR PERMIT	2025-12-09
G	ISSUED FOR TENDER-REV.01	2025-12-23

Arcadis Professional Services (Canada) Inc.

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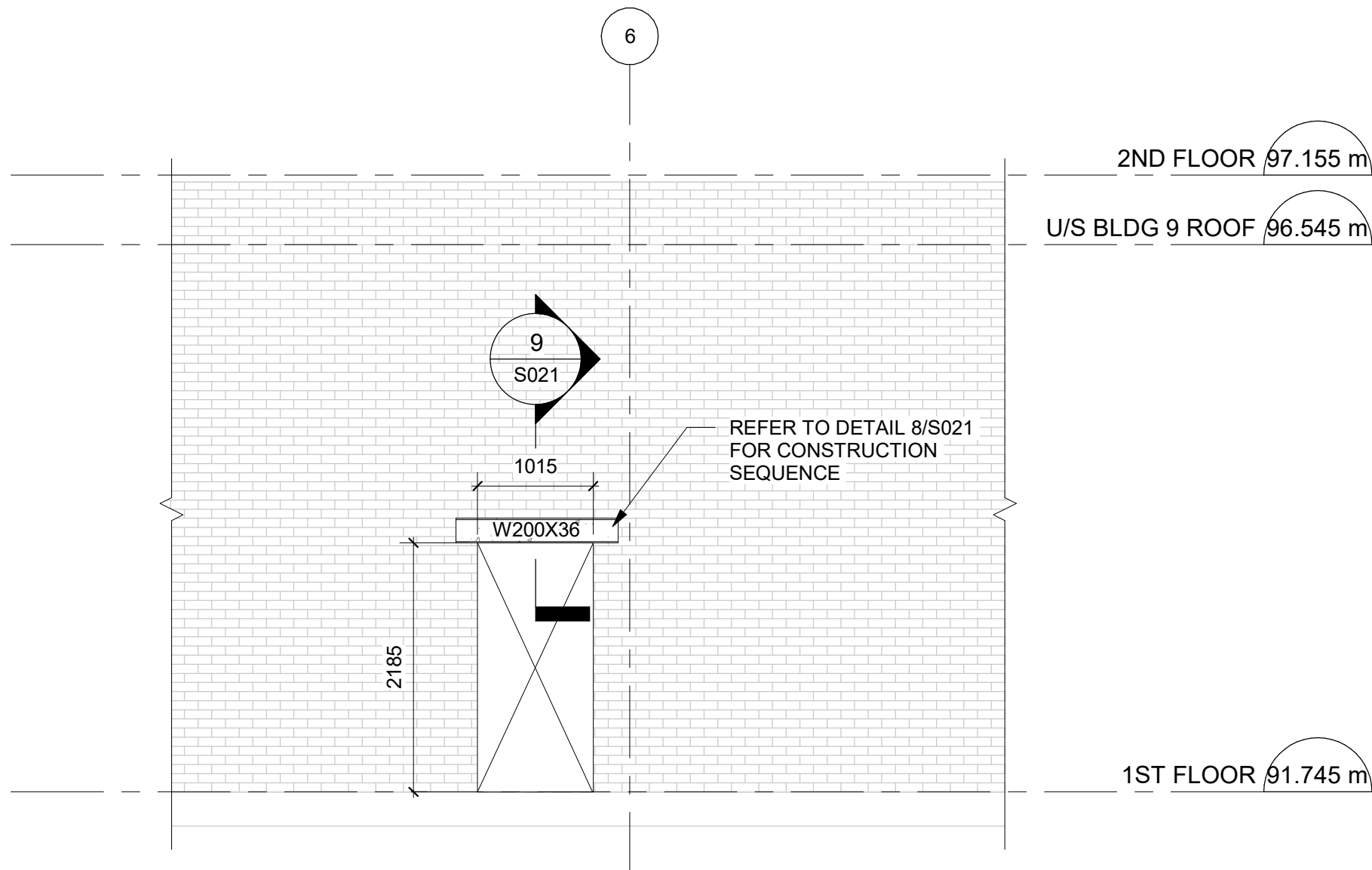
PROJECT TITLE
**TORONTO PARAMEDIC
SERVICES FLEET
MAINTENANCE STATION -
UPGRADE WORKS**

PROJECT ADDRESS
**KING STREET YARD -
BUILDING NO. 8 & 9
1116 KING STREET WEST**

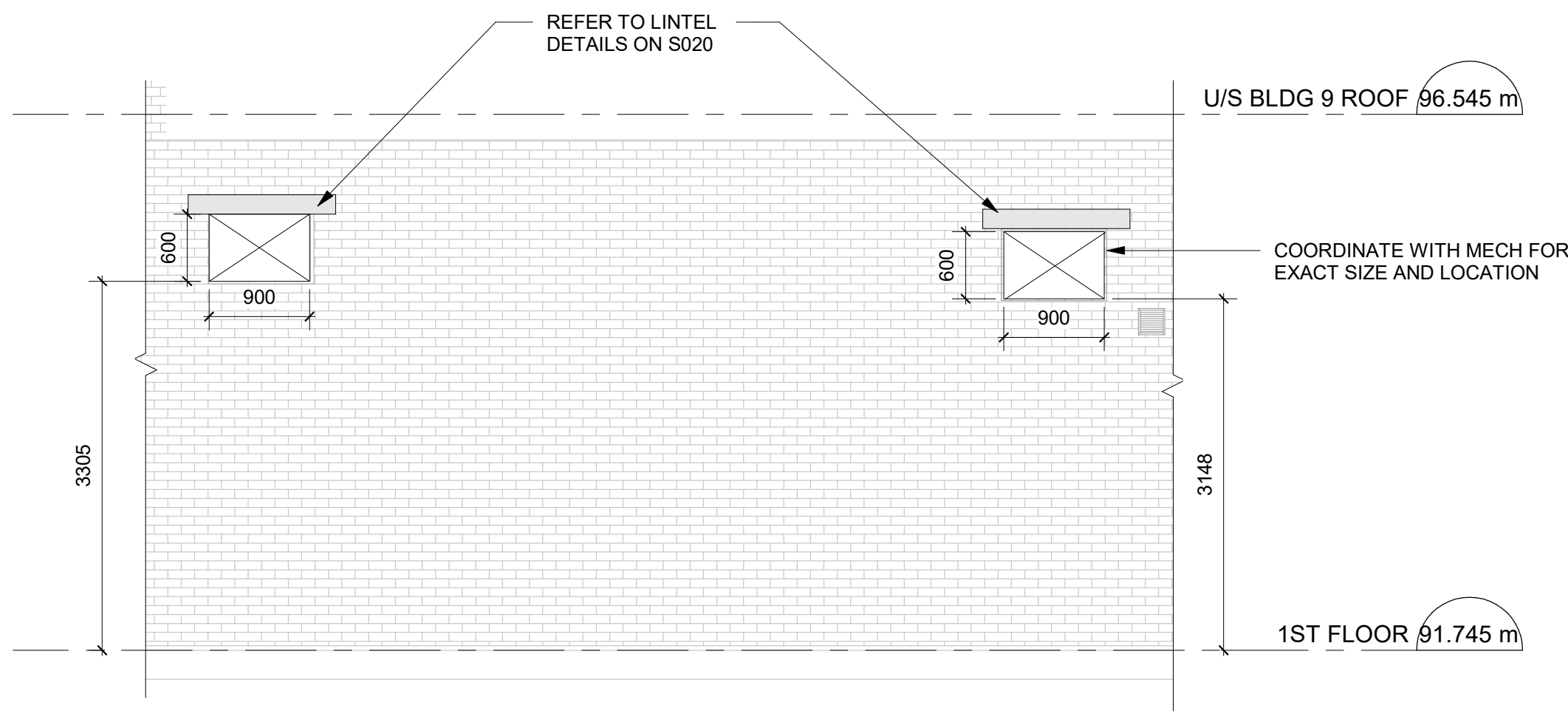
PROJECT NO:
30276606
DRAWN BY:
D. DU
PROJECT MGR:
N. LAYOUN
CHECKED BY:
R. CHHATRASHAL
APPROVED BY:
M. SHEININ

SHEET TITLE
WALL SECTIONS

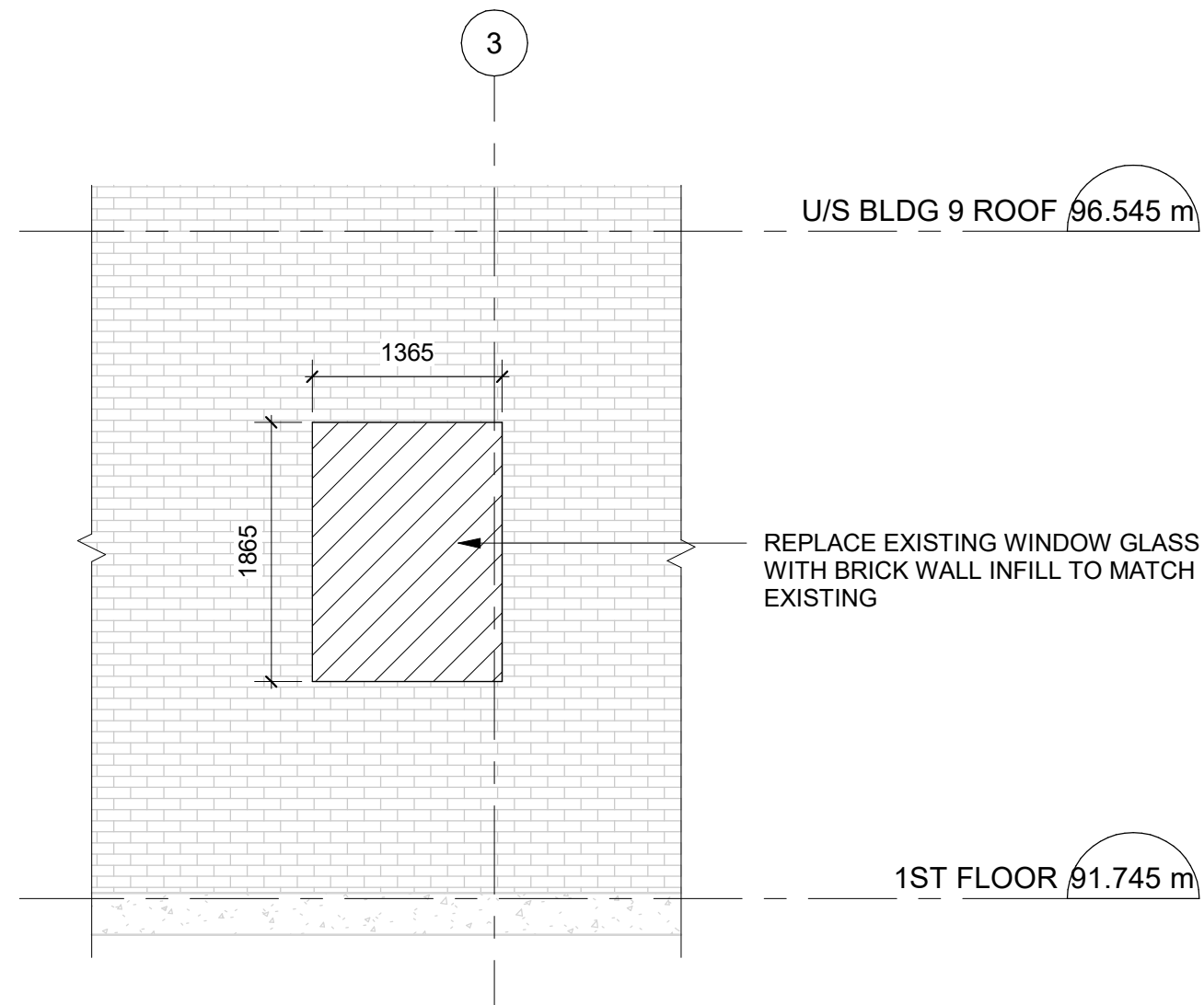
SHEET NUMBER
S301
ISSUE
G



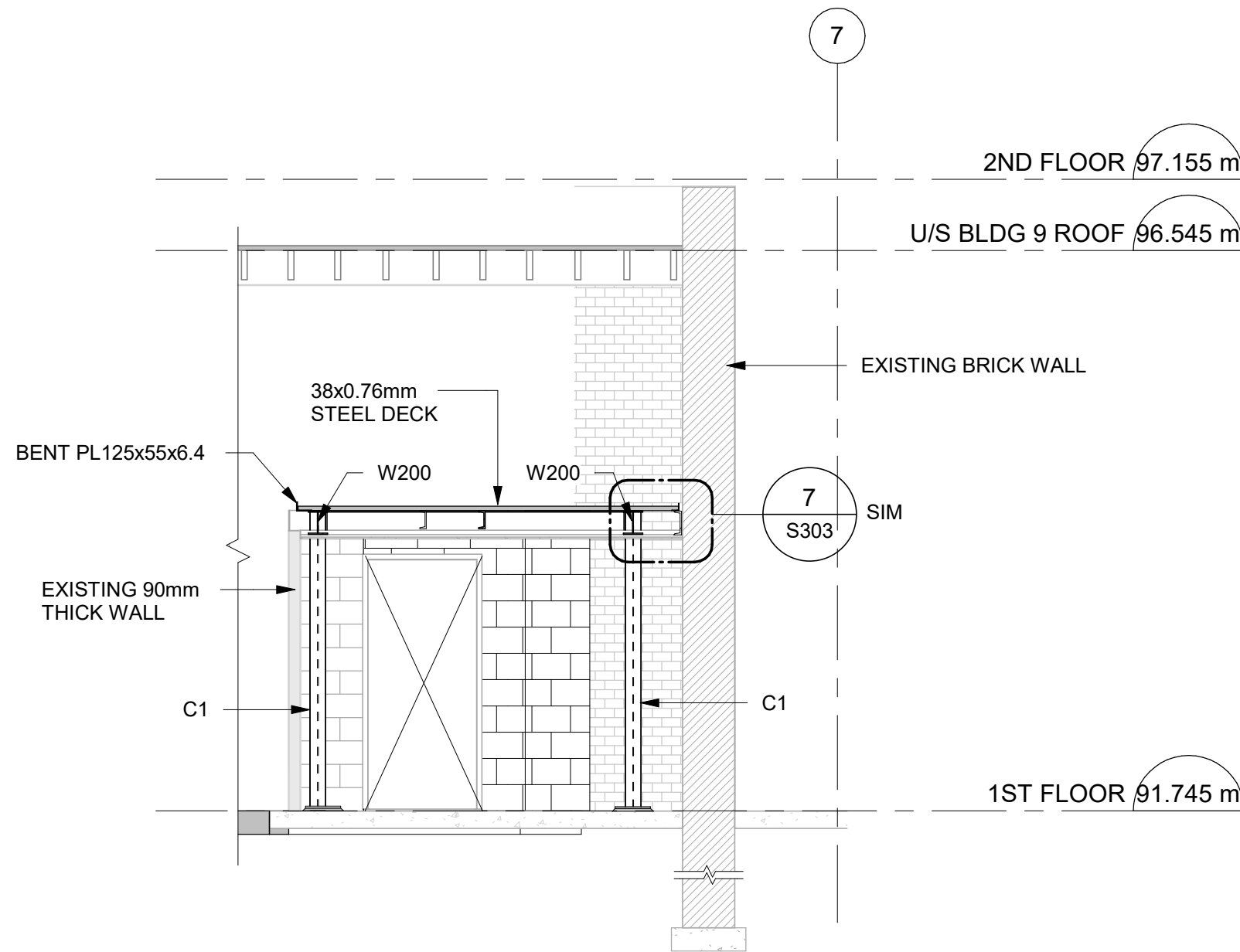
1 NEW DOOR OPENING - BUILDING 9
S302 Scale: 1 : 50



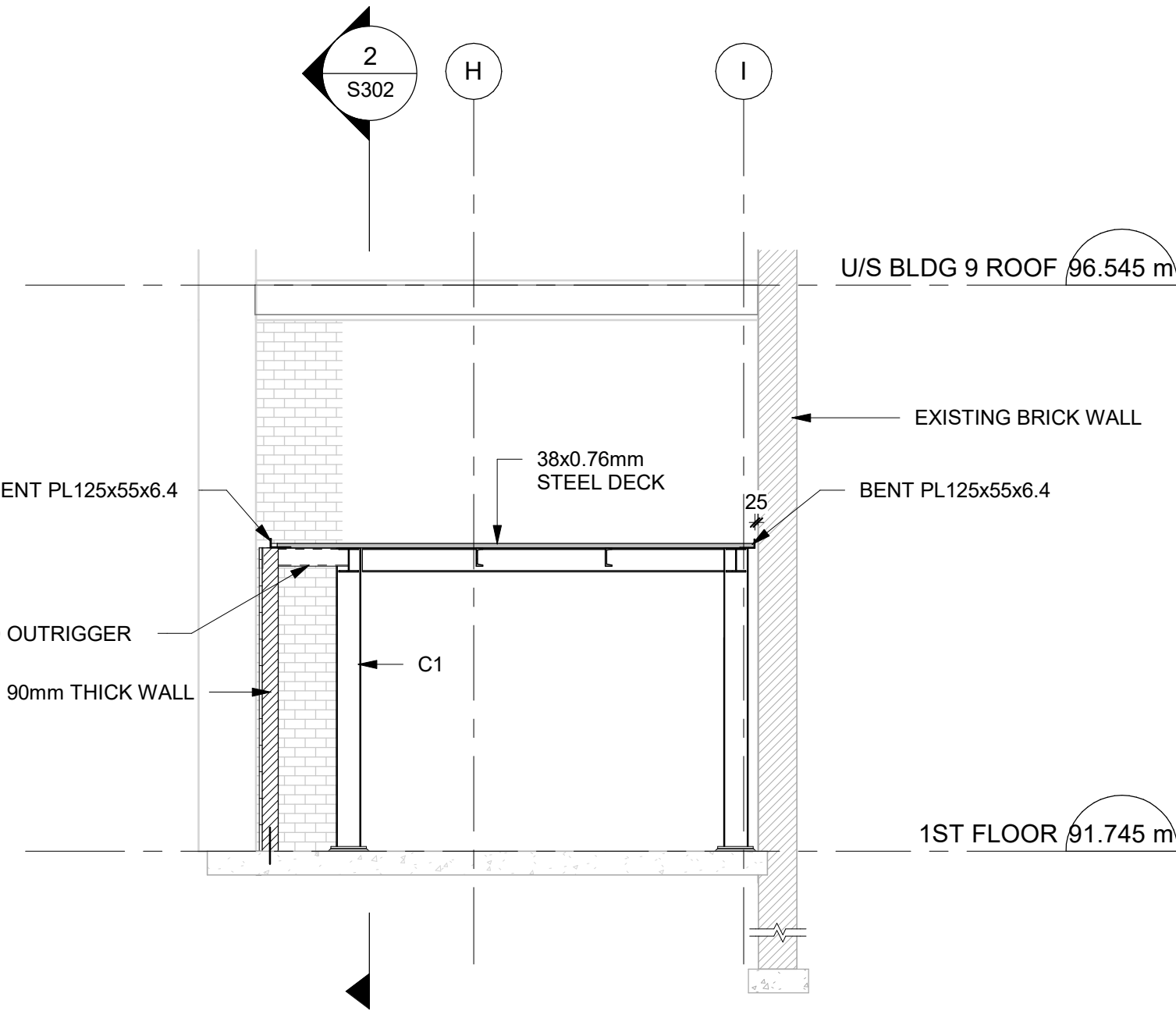
4 ELEVATION - OPENINGS ON EXISTING WALL
S302 Scale: 1 : 50



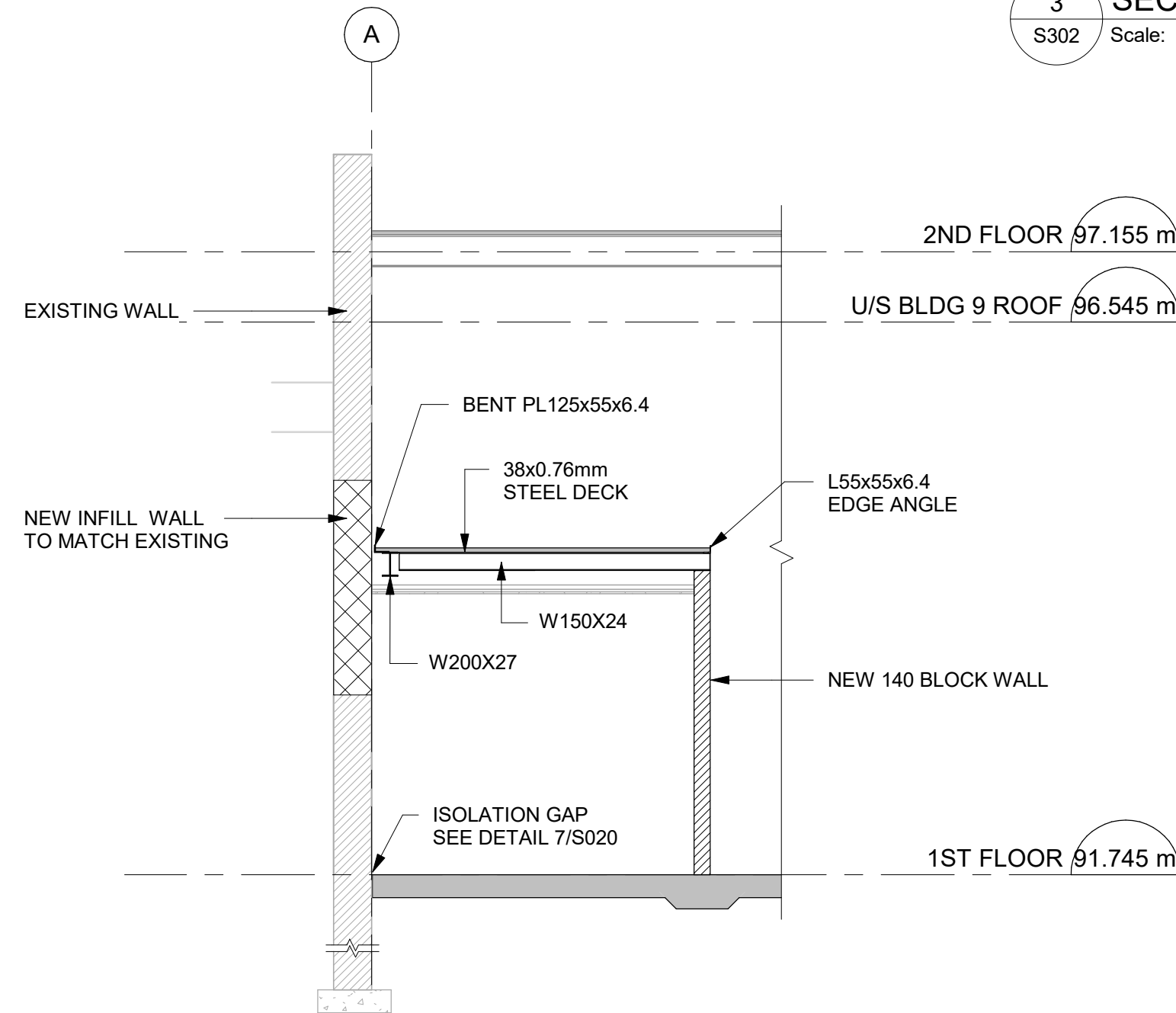
5 SECTION - WALL INFILL IN OFFICE ROOM 106
S302 Scale: 1 : 50



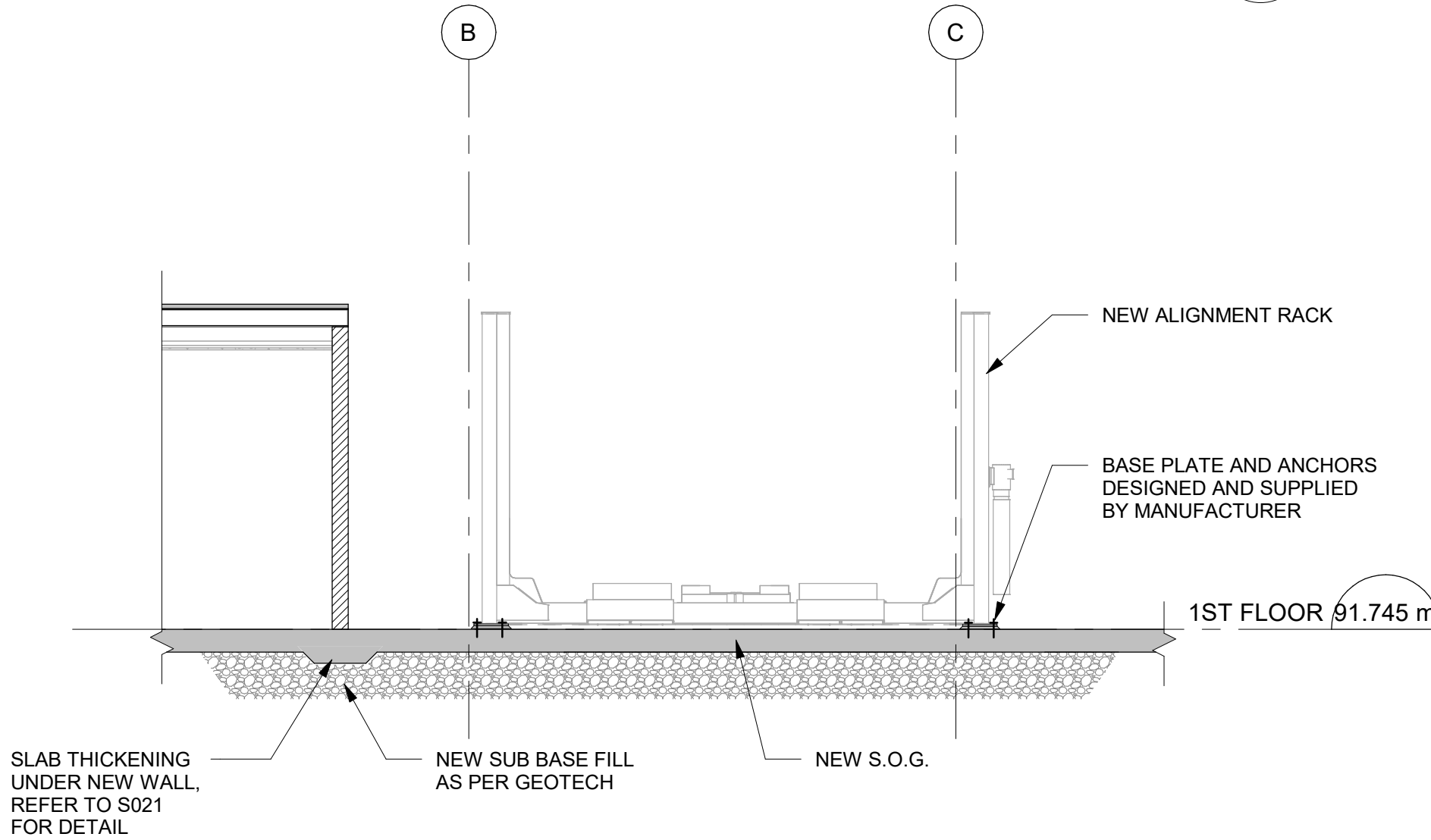
2 SECTION - BUILDING 9
S302 Scale: 1 : 50



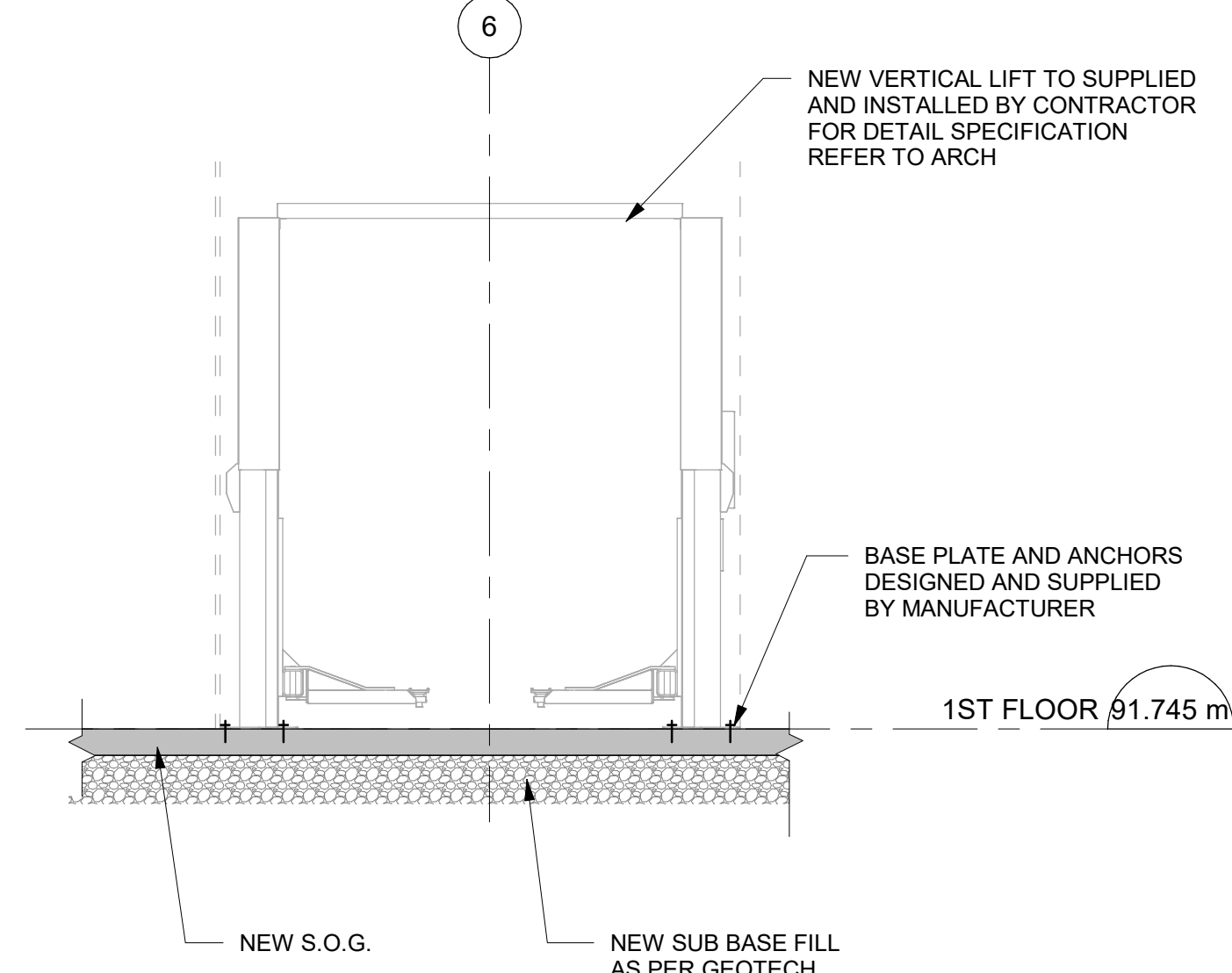
3 SECTION
S302 Scale: 1 : 50



6 SECTION
S302 Scale: 1 : 50



7 SECTION - ALIGNMENT RACK SUPPORTS
S302 Scale: 1 : 50



8 VERTICAL LIFT SECTION
S302 Scale: 1 : 50

ISSUES		
No.	DESCRIPTION	DATE
A	ISSUED FOR 50% REVIEW	2025-10-17
B	ISSUED FOR 75% REVIEW	2025-11-05
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E	ISSUED FOR TENDER	2025-12-09
F	ISSUED FOR PERMIT	2025-12-09
G	ISSUED FOR TENDER-REV.01	2025-12-23

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DRAWN BY:
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CHECKED BY:
R. CHHATRAHAL

PROJECT MGR:
N. LAYOUN

APPROVED BY:
M. SHEININ

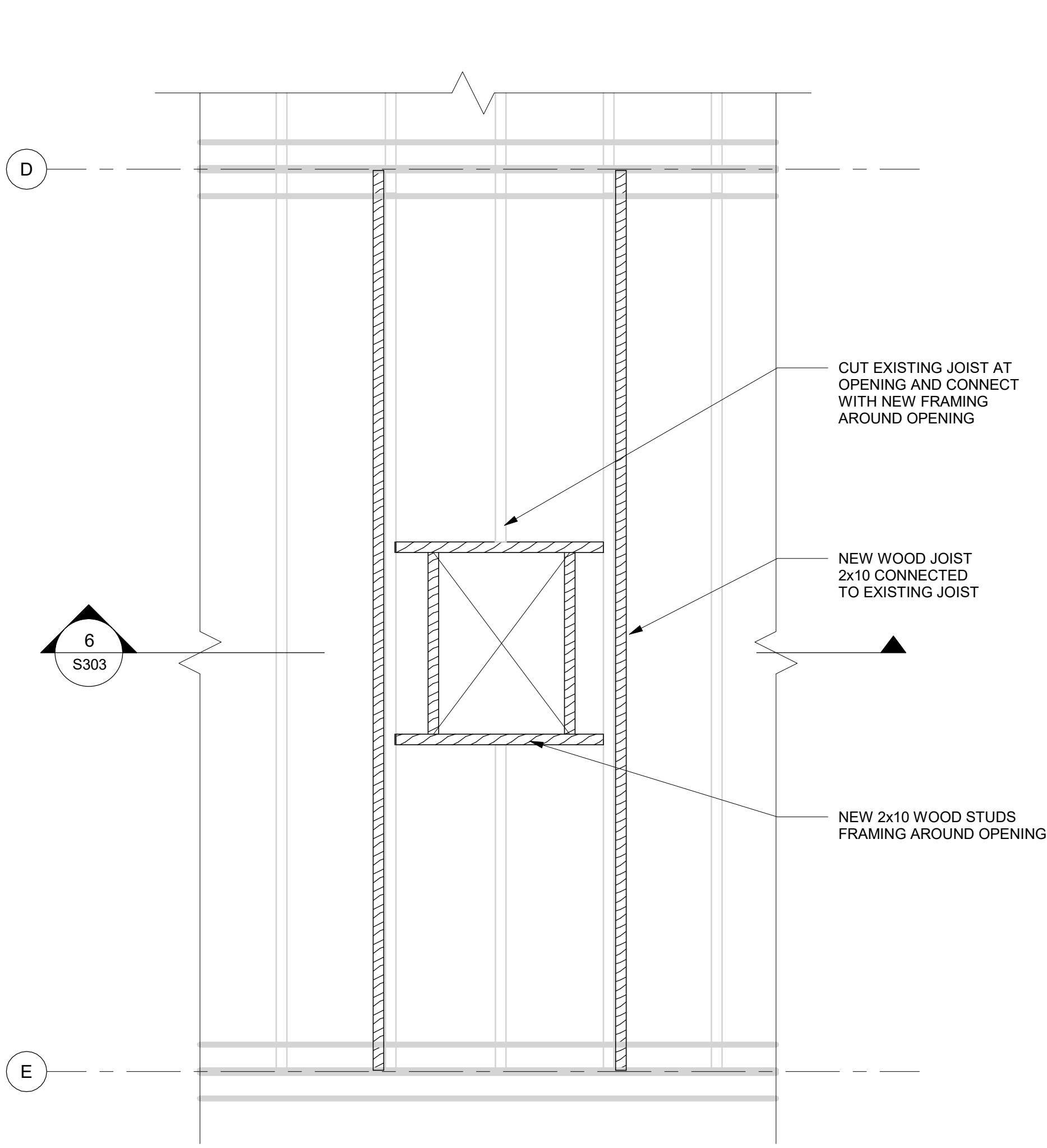
SHEET TITLE
SECTIONS AND DETAILS

SHEET NUMBER

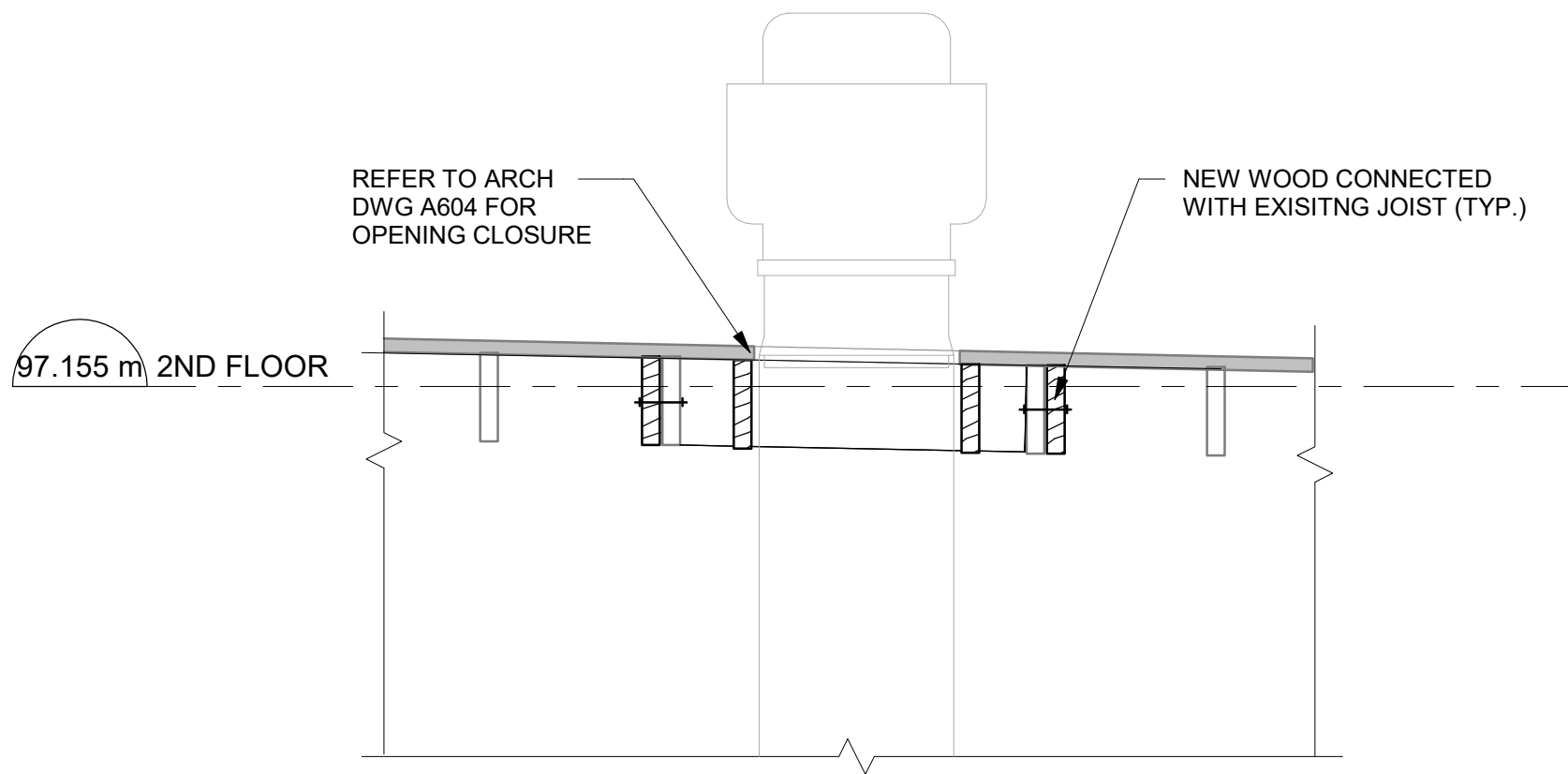
S302

ISSUE

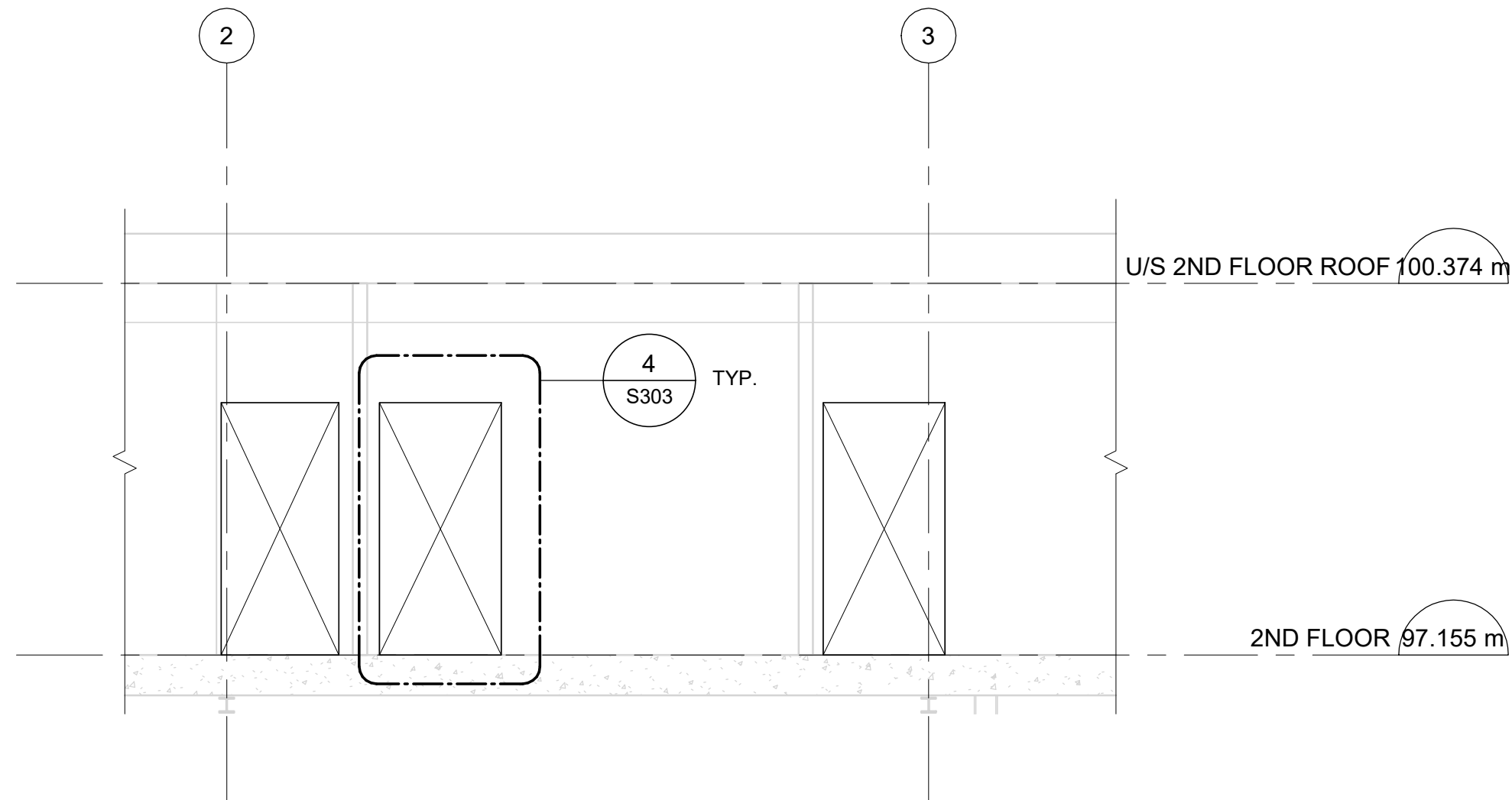
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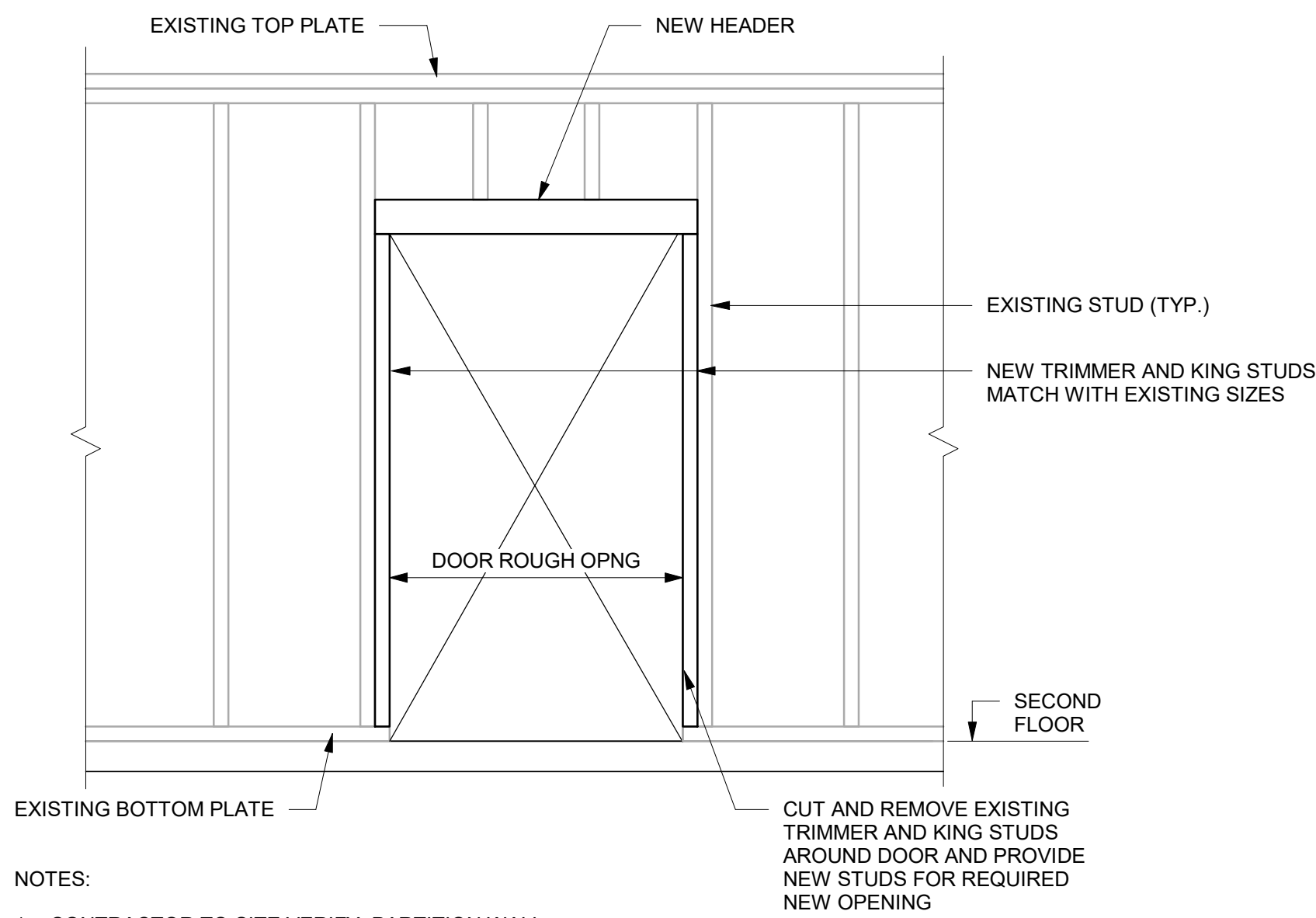
1 TYPICAL ROOF OPENING DETAIL
S303 Scale: 1 : 20



6 TYPICAL ROOF OPENING DETAIL
S303 Scale: 1 : 20

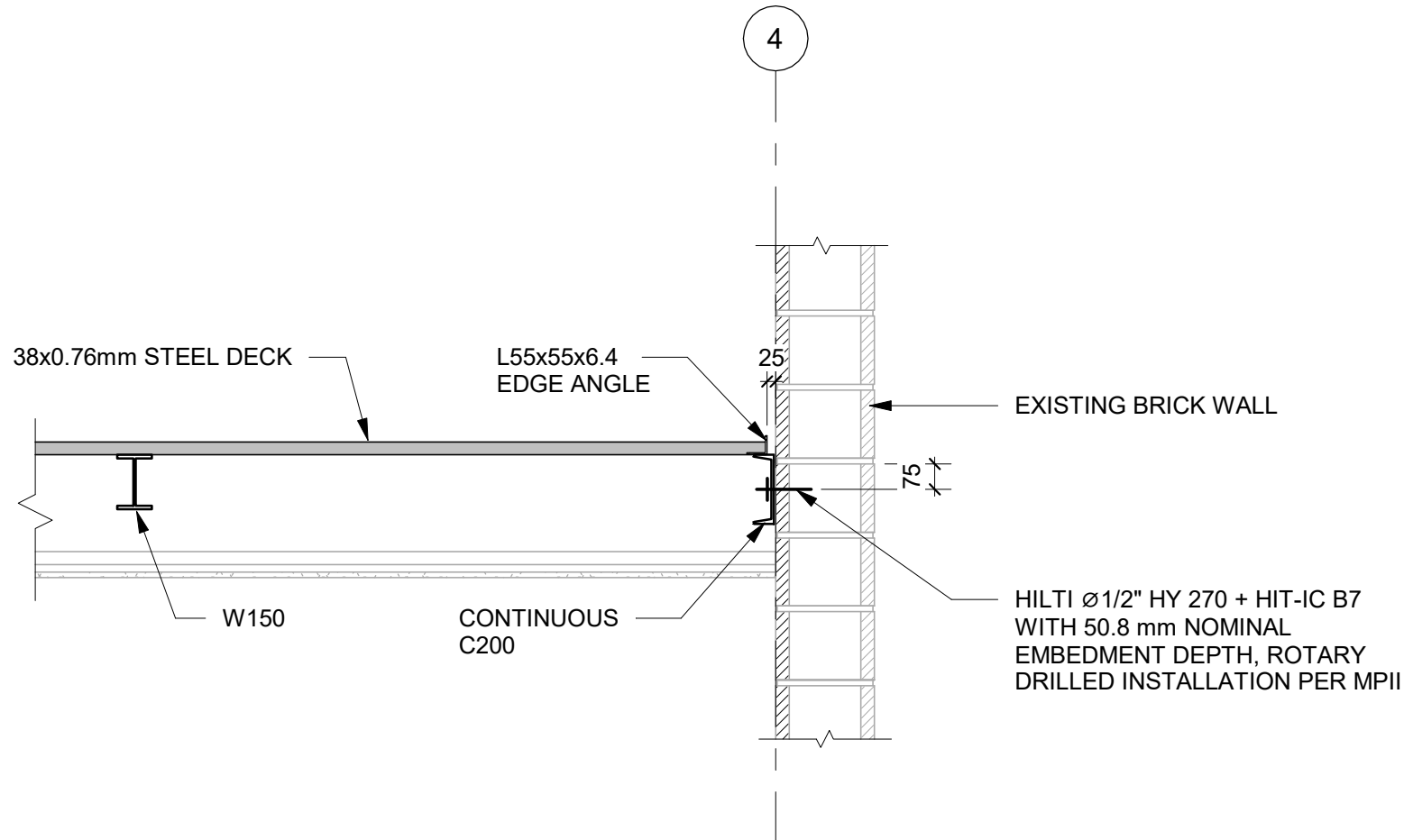


2 SECTION
S303 Scale: 1 : 50

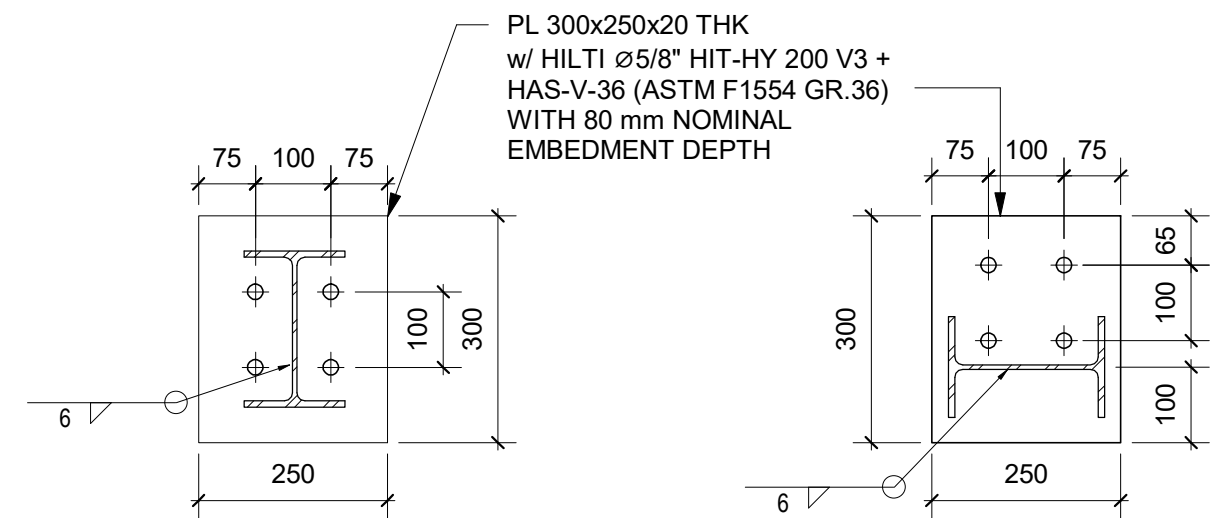


- NOTES:
- CONTRACTOR TO SITE VERIFY. PARTITION WALL IS NON LOAD BEARING BEFORE DEMOLITION AND FABRICATION.
 - DETAIL IS SHOWN SCHEMATICALLY FOR TENDER, ASSUMING STANDARD PRACTICE.
 - SUBMIT DETAIL SHOP DRAWING FOR WOOD WALL FRAMING MEMBER FOR DOORS AND ITS CONNECTIONS.

4 PARTITION WALL FRAMING
S303 Scale: 1 : 20



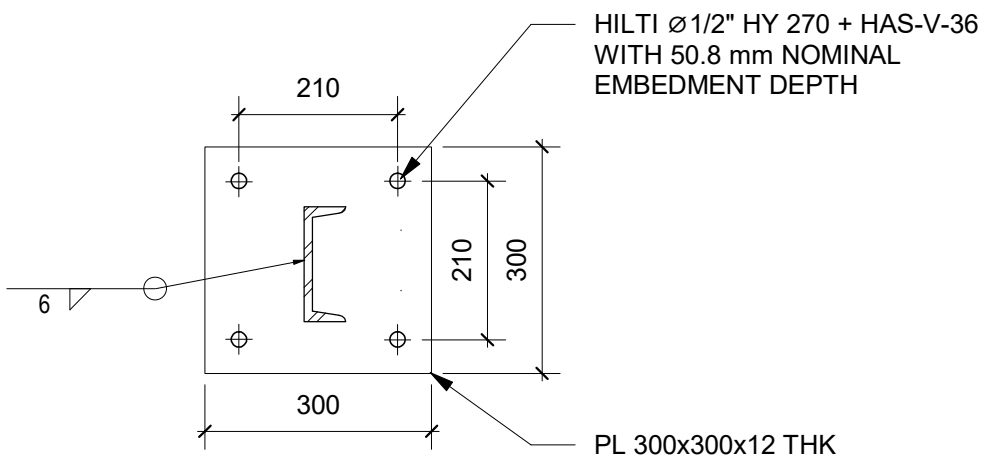
7 SECTION
S303 Scale: 1 : 20



BP1

BP2

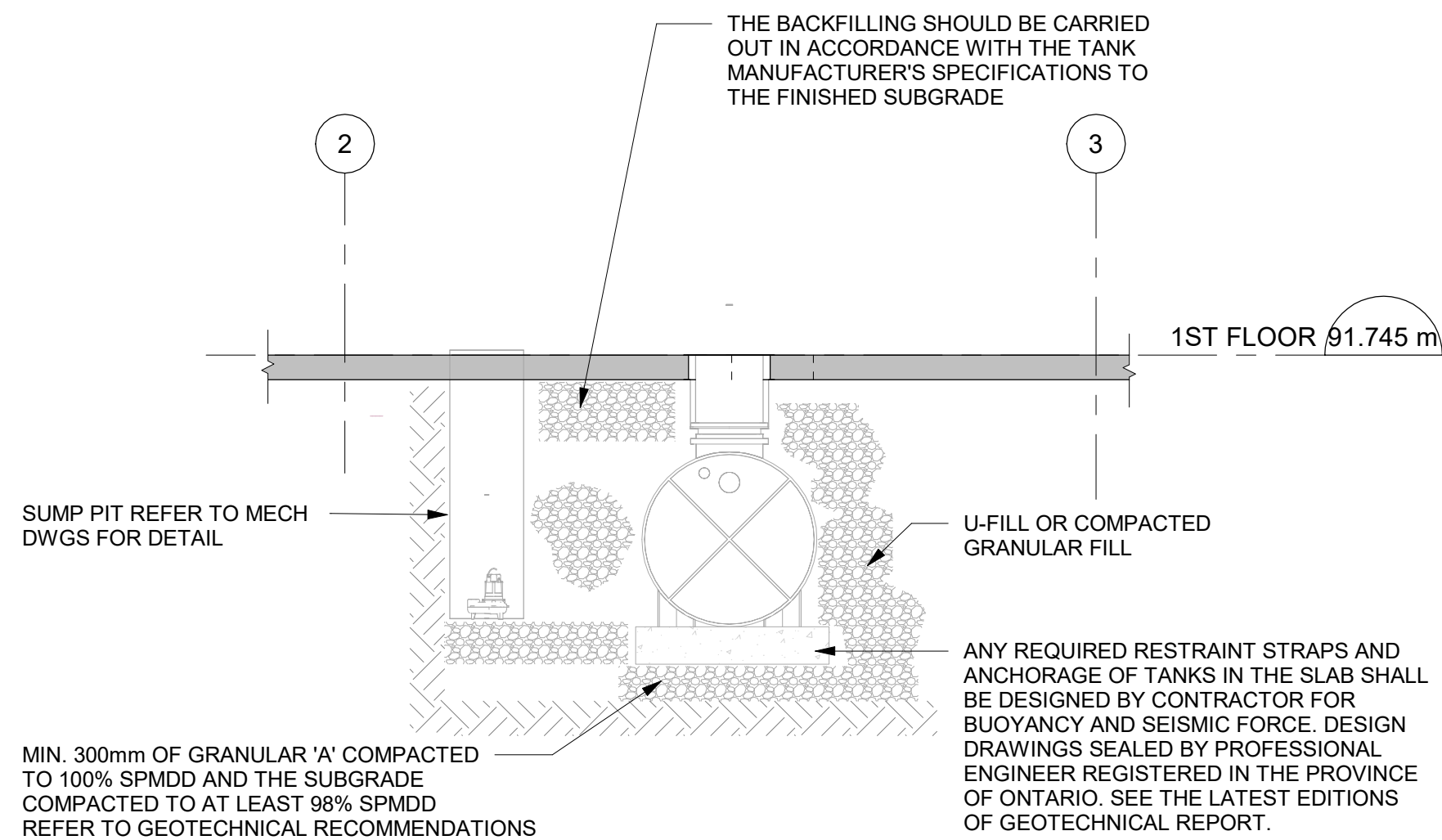
3 BASE PLATES
S303 Scale: 1 : 10



WP1

- NOTE:
- ANCHORS MUST BE MIN. 55 mm AWAY FROM MORTAR JOINTS

5 WALL PLATE
S303 Scale: 1 : 10



- NOTES:
- REFER TO MECHANICAL DRAWINGS FOR LOCATION, SIZE AND DETAILS.
 - CONTRACTOR TO PROVIDE SHORING AND DEWATERING AS REQUIRED TO ENSURE EXCAVATION STABILITY. SEE GEOTECHNICAL REPORT.
 - DIMENSION TO BE INSTALLED PER MANUFACTURER INSTRUCTIONS.
 - CONTRACTOR TO PROVIDE PRECAST CONCRETE SUMP PIT DETAIL DRAWINGS FOR CONSULTANT'S REVIEW. COORDINATE WITH MECHANICAL FOR SIZE, ELEVATION AND CONNECTION DETAILS.

8 SECTION - OIL INTERCEPTOR AND SUMP PIT
S303 Scale: 1 : 50

ISSUES		
No.	DESCRIPTION	DATE
A	ISSUED FOR 95% REVIEW	2025-11-28
B	ISSUED FOR 100% REVIEW	2025-12-09
C	ISSUED FOR TENDER	2025-12-09
D	ISSUED FOR PERMIT	2025-12-09
E	ISSUED FOR TENDER-REV.01	2025-12-23
F	ADDENDUM 3	2026-01-12

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SECTIONS AND DETAILS

SHEET NUMBER

S303

ISSUE

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